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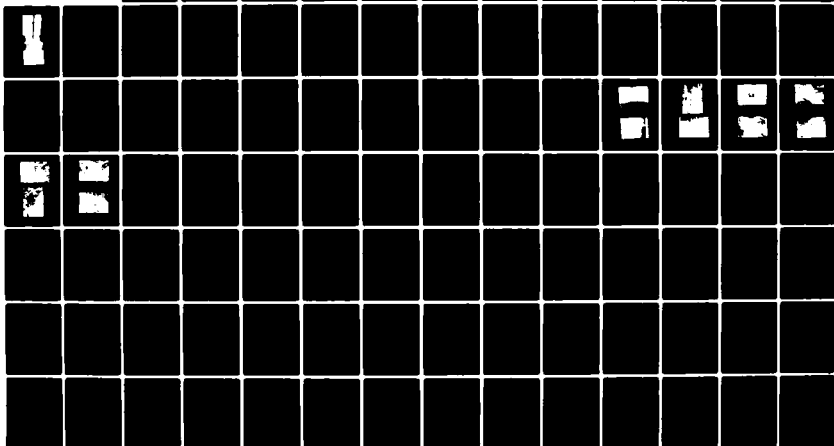
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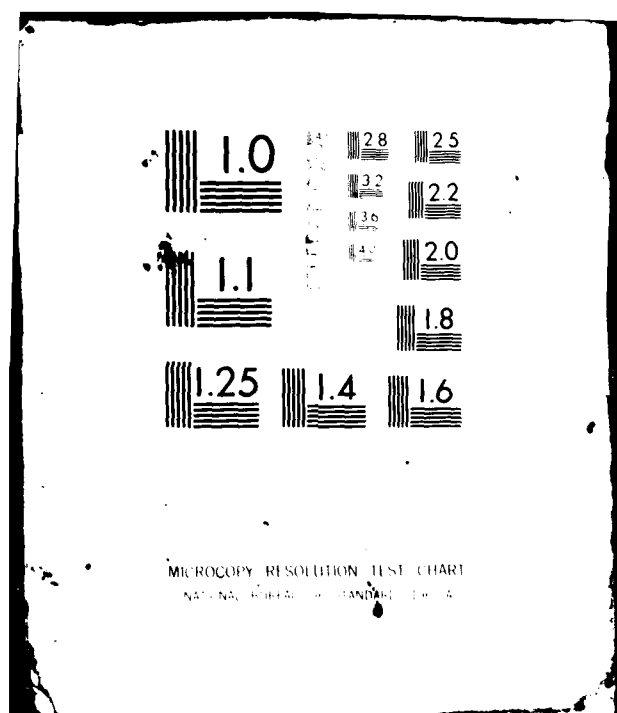
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LONG ISLAND BASIN

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LEVEL II

LONG POND DAM NO. 3

WESTCHESTER COUNTY, NEW YORK  
INVENTORY NO. N.Y. 115

PHASE I INSPECTION REPORT  
NATIONAL DAM SAFETY PROGRAM

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20. ABSTRACT (Continue on reverse side if necessary and identify by block number) This report provides information and analysis on the physical condition of the dam as of the report date. Information and analysis are based on visual inspection of the dam by the performing organization. Examination of available documents and a visual inspection of the dam and the appurtenant structures did not reveal conditions which constitute an immediate hazard to human life or property.		

Using Corps of Engineers' Screening Criteria, it has been determined that the dam would be overtopped for all storms exceeding approximately 41 percent of Probable Maximum Flood (PMF). The service spillway is, therefore, adjudged as "seriously inadequate" and the dam is assessed as unsafe, non-emergency.

The classification of "unsafe" applied to a dam because of a "seriously inadequate spillway" is not meant to connote the same degree of emergency as would be associated with an "unsafe" classification applied for a structural deficiency. It does mean, however, that based on an initial screening, and preliminary computations, there appears to be a serious deficiency in spillway capacity so that if a severe storm were to occur, overtopping and failure of the dam would take place, significantly increasing the hazard to loss of life downstream from the dam.

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**LONG ISLAND BASIN**

**LONG POND DAM NO. 3**

**WESTCHESTER COUNTY, NEW YORK  
INVENTORY NO. N.Y. 115**

**PHASE I INSPECTION REPORT  
NATIONAL DAM SAFETY PROGRAM**



**NEW YORK DISTRICT CORPS OF ENGINEERS**

**SEPTEMBER 1981**

## PREFACE

This report is prepared under guidance contained in the Recommended Guidelines for Safety Inspection of Dams, for Phase I Investigations. Copies of these guidelines may be obtained from the Office of Chief of Engineers, Washington, D. C., 20314. The purpose of a Phase I Investigation is to identify expeditiously those dams which may pose hazards to human life or property. The assessment of the general condition of the dam is based upon available data and visual inspections. Detailed investigations, and analyses involving topographic mapping, sub-surface investigations, testing, and detailed computational evaluations are beyond the scope of a Phase I Investigation; however, the investigation is intended to identify any need for such studies.

In reviewing this report, it should be realized that the reported condition of the dam is based on observations of field conditions at the time of inspection along with data available to the inspection team. In cases where the reservoir was lowered or drained prior to inspection, such action, while improving the stability and safety of the dam, removes the normal load on the structure and may obscure certain conditions which might otherwise be detectable if inspected under the normal operating environment of the structure.

It is important to note that the condition of a dam depends on numerous and constantly changing internal and external conditions, and is evolutionary in nature. It would be incorrect to assume that the present condition of the dam will continue to represent the condition of the dam at some point in the future. Only through frequent inspections can unsafe conditions be detected and only through continued care and maintenance can these conditions be prevented or corrected.

Phase I inspections are not intended to provide detailed hydrologic and hydraulic analyses. In accordance with the established Guidelines, the Spillway Test flood is based on the estimated "Probable Maximum Flood" for the region (greatest reasonably possible storm runoff), or fractions thereof. Because of the magnitude and rarity of such a storm event, a finding that a spillway will not pass the test flood should not be interpreted as necessarily posing a highly inadequate condition. The test flood provides a measure of relative spillway capacity and serves as an aid in determining the need for more detailed hydrologic and hydraulic studies, considering the size of the dam, its general condition and the downstream damage potential.



PHASE I INSPECTION REPORT  
NATIONAL DAM SAFETY PROGRAM  
LONG POND DAM NO. 3  
I.D. NO. N.Y. 00115  
D.E.C. NO.  
LONG ISLAND BASIN  
WESTCHESTER COUNTY, NEW YORK

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PHASE I INSPECTION REPORT  
NATIONAL DAM SAFETY PROGRAM

NAME OF DAM: Long Pond Dam No. 3, N.Y. 00115  
STATE LOCATED: New York  
COUNTY LOCATED: Westchester  
STREAM: Mianus River  
BASIN: Long Island  
DATE OF INSPECTION: June 9, 1981

ASSESSMENT

Examination of available documents and a visual inspection of the dam and the appurtenant structures did not reveal conditions which constitute an immediate hazard to human life or property.

Using Corps of Engineers' Screening Criteria, it has been determined that the dam would be overtopped for all storms exceeding approximately 41 percent of Probable Maximum Flood (PMF). The service spillway is, therefore, adjudged as "seriously inadequate" and the dam is assessed as unsafe, non-emergency.

The classification of "unsafe" applied to a dam because of a "seriously inadequate spillway" is not meant to connote the same degree of emergency as would be associated with an "unsafe" classification applied for a structural deficiency. It does mean, however, that based on an initial screening, and preliminary computations, there appears to be a serious deficiency in spillway capacity so that if a severe storm were to occur, overtopping and failure of the dam would take place, significantly increasing the hazard to loss of life downstream from the dam.

It is, therefore, recommended that within 3 months of notification to the owner, detailed hydrological-hydraulic investigations of the structure should be undertaken to more accurately determine the site specific characteristics of the watershed and their affect upon the overtopping potential of the dam. At the same time a dam break analysis should be carried out to ascertain the effect of a sudden flood from the upstream reservoir discharging into the lower reservoir. The results of these investigations will determine the appropriate remedial measures which will be required to achieve a spillway capacity

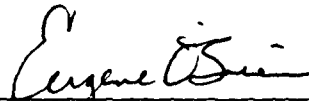
adequate to discharge the outflow from at least the  $\frac{1}{2}$  PMF.

In the interim, a detailed emergency action plan must be developed and implemented during periods of unusually heavy precipitation. Also, around-the-clock surveillance of the structure must be provided during these periods.

The following remedial measures must be completed within one year:

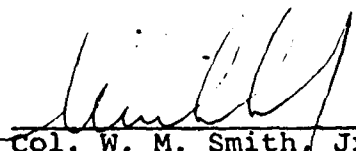
1. Nature of the seepage at the central toe of the dam should be investigated.
2. Location of the reservoir drain discharge should be determined.
3. Only after locating the reservoir drain discharge, the workability of the drain valve should be evaluated. If found to be inoperable or defective, it should be repaired to a good working condition or replaced.
4. The concrete on the spillway training walls and downstream bottom slab should be repaired.
5. The rocklined spillway discharge channel should be evaluated to determine if it has sufficient capacity to contain calculated maximum spillway flows.
6. The seepage area at the south abutment should be blanketed with a properly filtered drainage material to allow controlled transportation of seepage waters downslope in addition to minimizing the potential for piping of finer grained embankment soils.
7. All small trees, dead large trees and large trees located near the crest and upstream slope should be cut down and removed. Larger trees on the downstream slope should be inventoried and their condition monitored. If one of these trees dies it should be cut down and the area around the stump should be monitored for the development of seepage. Holes and depressions developing as a result of tree removal should be backfilled and sealed.
8. The low lying area, upstream of Duck Pond immediately adjacent to the dam's toe should be backfilled to prohibit backwater encroachment on the downstream slope.
9. The upstream slope of the dam should be properly protected against erosion.

10. A program of periodic inspection and maintenance of the dam and appurtenances including yearly operation and lubrication of the repaired gates should be provided. This program should be documented for future reference. The emergency action plan, described in Section 7.1d, should be maintained and updated periodically during the life of the structure.



Eugene O'Brien, P.E.  
New York No. 29823

Approved By:



Col. W. M. Smith, Jr.  
New York District Engineer

Date:





1. GENERAL OVERVIEW OF DAM

PHASE I INSPECTION REPORT  
NATIONAL DAM SAFETY PROGRAM  
LONG POND DAM NO. 3  
I.D. NO. N.Y. 00115  
D.E.C. NO.  
LONG ISLAND BASIN  
WESTCHESTER COUNTY, NEW YORK

SECTION 1 - PROJECT INFORMATION

1.1 GENERAL

a. Authority

The Phase I inspection reported herein was authorized by the Department of the Army, New York District, Corps of Engineers by Contract No. DACW 51-81-C-0008, dated 14 December 1980, in fulfillment of the requirements of the National Dam Inspection Act, Public Law 92-367, 8 August 1972.

b. Purpose of Inspection

The inspection was conducted to evaluate the existing conditions of the dam, to identify deficiencies and hazardous conditions, to determine if these deficiencies constitute hazards to life or property, and to recommend remedial measures where required.

1.2 DESCRIPTION OF THE PROJECT

a. Description of Dam and Appurtenant Structures

The Long Pond Dam No. 3 is composed of a 355 ft long earth and rockfill embankment having a maximum height of approximately 40 ft and crest width of 34 ft. Upstream slopes of the dam typically range from 1 Vertical to 5 Horizontal (1V:5H) to 1V:6H. In the vicinity of the spillway approach upstream slopes are steeper, on the order of 1V:3H. No slope protection is present on the upstream face. The downstream slope typically has a 1V:1.5-1.0H slope descending about 20 ft from the crest to a 20 ft wide horizontal bench and then continuing



downslope at a 1V:1.5-2.0H slope to the toe. Near the north abutment in the vicinity of the rocklined spillway channel, downstream slopes are typically 1V:3H. Typical slope geometry is shown in section view in Appendix A.

The spillway, located at the north abutment contact in a reinforced concrete box culvert 4 ft high and 6 ft wide. Base slab elevation of the spillway is about 6 ft below the dam crest. The spillway is uncontrolled and passes water through the dam crest discharging into a rocklined channel running down the north abutment.

The dam which creates a recreational lake is equipped with a reservoir drain of unknown dimension and construction. Gatehouse for this drain is located about 100 ft upstream of the center of the dam (See Photograph 6).

North Lake Dam is situated approximately 1,200 ft upstream of Long Pond Dam. An uncontrolled overflow spillway passes water from North Lake directly into Long Pond.

b. Location

Long Pond No. 3 Dam is located at the south end of Long Pond Court within the Windmill Farms Development, approximately 6½ miles northeast of the City of North Castle, Westchester County, New York.

c. Size Classification

The dam is 40 ft high and has a reservoir at this height with a storage capacity of approximately 115 acre-feet and is therefore classified as a small dam.

d. Hazard Classification

The dam is in the "high" hazard potential category because it is located upstream of a moderately to densely populated area of the Windmill Farms subdivision.

e. Ownership

The Long Pond Dam No. 3 is located on property owned by Mr. K. Karl Mueller, 4 Long Pond Court, Windmill Farms, Armonk, N.Y. 10504, Telephone No.: (914) 273-8074. However, the lake bottom property and the reservoir drain control house is owned by the Town of New Castle.

f. Purpose of Dam

The dam was constructed to create a lake used for recreational purposes.

g. Design and Construction History

Design drawings or construction records do not exist for the Long Pond Dam No.3. The dam was reportedly designed by Elwyn E. Seelye and Company, Park Avenue, New York, N.Y. and built circa 1936.

h. Normal Operating Procedure

Water periodically flows through the uncontrolled spillway. It is not known if the reservoir drain is periodically used for water level control or even operational.

1.3 PERTINENT DATA

a. <u>Drainage Area</u> (sq. miles)	0.46
b. <u>Discharge at Dam Site</u> (cfs)	
Ungated Spillway at Maximum Pool	222
Capacity of Reservoir drain	unknown
Total Discharge, Maximum Pool (El. 476)	222 $\pm$
c. <u>Elevation</u> (ft above MSL USGS Datum) *	
Top of Dam	476 $\pm$
Maximum Design Pool	474 $\pm$
Spillway, Upstream Invert	470 $\pm$
Inverts, Reservoir Drain	unknown

\* All elevations are based on pond level datum relative to an elevation interpolated from the USGS QUAD sheet.

d. Reservoir

Length of Maximum Pool (miles)	0.096
Length of Shoreline at Spillway	
Crest (miles)	0.74
Surface Area (acres)	11.28

e. Storage (acre-feet)

Reservoir at Spillway Crest	115
Reservoir at Maximum Pool	173.6

f. Dam

Type	Earth and Rock Fill
Length (feet)	355 ft
Height (feet)	40
Upstream Slope	1V:5H
Downstream Slope	1V:1.5-2.0H w/ 20 ft bench at mid- height
Crest Elevation (feet)	476 *
Crest Width (feet)	34
Cutoff Type	None
Grout Curtain	None

g. Spillway

Type	Concrete, 4' x 6' box culvert with rocklined open channel running down north abutment
Length (feet)	54
Crest Elevation (feet)	470 *

h. Reservoir Drain and Pipeline

A control structure, located approximately at the center of the dam, and about 100 ft upstream, houses the control valves for the reservoir drain. Valve type, pipe size and

\* See Note on Page 3.

location of discharge are unknown, and/or could not accurately be determined at the time of inspection. Access to the off-shore gate house was not possible during inspection, therefore, the condition or workability of the drain control could not be ascertained.

## SECTION 2 - ENGINEERING DATA

### 2.1 GEOLOGY

The records of the owner contain no data on site geology. However, there is data available in the published literature (Ref.4) on the general geology of the area. Long Pond Dam. No.3 is located in the New England Upland Section of the New England Maritime Physiographic Province. The bedrock in this section consists of metamorphic, igneous and sedimentary rocks which have undergone a complex sequence of deposition, folding, faulting and erosion. In the vicinity of the damsite, bedrock is primarily composed of Fordham gneiss.

### 2.2 SUBSURFACE INVESTIGATION

There is no record of subsurface investigation for the dam. The shallow surface soils in the area are of glacial origin and for the most part consist of complex sands, silts, gravels and boulders.

### 2.3 DAM AND APPURTENANT STRUCTURES

The files of the owner contain no prints, design data or construction record of the dam. There are no drawings of the outlet works or any subsequent changes which may have been made.

### 2.4 CONSTRUCTION RECORDS

No information has been located in relation to the construction of the dam, spillway or outlet works. Based on the construction history of several dams built in conjunction with development of the Windmill Farms Area, the completion of the project was probably circa 1936.

### 2.5 OPERATION RECORDS

According to the owner there are no records of operation for the dam.

### 2.6 EVALUATION OF DATA

The data available in the records of the owner, along with a field inspection and personal interviews are sufficient to support a Phase I evaluation of the dam.

## SECTION 3 - VISUAL INSPECTION

### 3.1 FINDINGS

#### a. General

The visual inspection of the Long Pond Dam No. 3 was performed on June 9, 1981. The weather was overcast with intermittent showers and temperatures ranges in the mid to high 70's F. The reservoir was at a level approximately 5.5 ft below the crest of the dam, about 6 inches deep in the spillway.

#### b. Dam

The dam is generally in good condition, showing no noticeable signs of vertical or horizontal displacements. Slopes appear to be stable. Seepage at the south abutment contact at a point mid-height on the slope was observed. Quantities of seepage were relatively small and soil transport did not appear to be occurring; however, this seepage still requires immediate attention. There is no slope protection on the upstream face of the dam, however, no significant erosion was observed.

Additionally, the following adverse conditions were noted:

1. There are a great number of very large trees growing on the downstream slope of the dam. Near the crest of the dam the root systems are likely to extend to the upstream face of the dam. This could result in the formation of seepage paths through the dam.

2. There is no emergency action plan for the project.

#### c. Spillway

The reinforced box culvert spillway, located at the north abutment, is in fair condition. Minor spalling of the concrete approach walls was observed. Of major concern, is the

broken and displaced slab at the downstream end of the spillway. A majority of the water passing through the spillway presently passes and exits below the downstream slab which, as a result is severely undermined (See Photograph 8 ). The spillway channel located along the north abutment is heavily revetted, with massive boulders (See Photograph 9 ) and appears in good condition for the present amount of overflow. There is no definite shape or rigid alignment to the channel and hence it may tend to extend onto unprotected portions of the embankment when required to handle large quantities of flow.

d. Appurtenant Structures

Condition, description and workability of the upstream reservoir drain controls are not known. Location of the downstream exit of the reservoir drain could not be positively identified, but is suspected to be partially buried within a pile of rock near the central downstream toe of the dam. Flowing water from this area may indicate that the valve is partially open or leaking.

e. Downstream Channel

The spillway tailrace channel passes along the north abutment of the dam and exits in the natural channel of the Mianus River.

f. Abutments

The abutment-dam contact areas are generally in good condition with the exception of a small seep located about mid-slope at the south abutment.

g. Reservoir Area

In the general vicinity of the dam and along the south of the lake, reservoir slopes are generally stable. No slides, rock falls or sloughing was observed during the inspection. There are however, two unusual conditions in the reservoir area which are considered significant. These are:

1. A portion of Long Pond road, built on a low embankment, forms an estimated 600 to 800 ft long stretch of reservoir shoreline at the southeast end of the lake. At its present level, the lake does not immediately infringe on the embankment, however, during flood stage, this portion of the roadway embankment will act as a dam. Based on "rough" measurements made during the dam inspection visit, it is estimated that a majority of this 600 to 800 ft long stretch of Long Pond road is 1 to 1½ ft lower in elevation than the crest of Long Pond Dam No. 3 and consequently may act as a secondary spillway under severe flood conditions.

2. The close proximity of North Lake Dam is located immediately upstream of the reservoir.

### 3.2 EVALUATION OF OBSERVATIONS

Although deficiencies were observed, there is no indication that the dam is in imminent danger. Some of the deficiencies observed in the previous paragraphs are minor and should be corrected by the owners' maintenance forces. Other conditions described above, however, represent conditions which may have potential for further deterioration and for this reason these conditions need to be further investigated and corrected.

The following is a summary of the problem areas encountered requiring further investigations and corrective action. An appropriate recommended action is included.

1) Nature of the flowing water emitting from the rockpile at the central toe area of the dam should be determined. This may be the location of the reservoir drain outlet.

2) Condition and workability of the reservoir drain controls should be evaluated.

3) All small trees (trunk less than 8 inches in diameter), large dead trees and large trees located near the crest of the dam should be removed with the resulting holes or depressions properly backfilled and seeded. Larger trees on the downstream slope should be inventoried and their conditions monitored.



4) Seepage area at the south abutment should be blanketed with a graded gravel to mitigate surface water erosion or potential piping. This area should also be monitored on a frequent basis.

5) The downstream end of the spillway should be repaired including backfilling undercut areas.

6) A program of periodic inspection and maintenance of the dam and appurtenances, including yearly operation and lubrication of the moving parts of the reservoir drain should be provided. This program should be documented for future reference. The aforementioned emergency action plan should be maintained and updated periodically during the life of the structure.

## SECTION 4 - OPERATION AND MAINTENANCE PROCEDURES

### 4.1 PROCEDURES

No written operation and maintenance procedures exist for the facility. The normal operation is to allow uncontrolled flow through the spillway.

### 4.2 MAINTENANCE OF DAM

It is reported that no routine maintenance of the dam is performed.

### 4.3 WARNING SYSTEM IN EFFECT

No warning system is in effect or in preparation.

### 4.4 EVALUATION

The overall operation and maintenance of the Long Pond Dam No. 3 is considered inadequate as a result of the following conditions:

1. Inaccessible or otherwise buried reservoir drain discharge
2. Deteriorating spillway
3. Uncontrolled seepage at the south abutment
4. Absence of a written operation and maintenance procedure
5. Absence of any written maintenance history

## SECTION 5 - HYDROLOGIC/HYDRAULICS

### 5.1 DRAINAGE AND CHARACTERISTICS

The Long Pond Dam is located in North Castle Township, Westchester County, New York (Hydrologic Unit Code No. 01100006). The area contributing to Long Pond is 0.52 sq. miles, and includes two reservoirs, North Lake and Windmill Lake. The basin is hilly with steep wooded slopes and narrow valleys. North Lake controls 0.26 sq. miles of the drainage area, while Windmill Lake controls 0.06 sq. miles (40.4 acres).

### 5.2 ANALYSIS CRITERIA

Spillway capacity adequacy was analysed by developing a design flood, using the unit hydrograph and the Probable Maximum Precipitation (PMP). For the analyses, the drainage area was divided into four sub-areas. The runoff from the drainage area contributing to Windmill Lake was assumed to be totally controlled by the available storage of the lake and was not included. However, the watershed downstream of Windmill Lake - subarea 2 - was assumed to contribute to Long Pond. Runoff from the North Lake sub-area was routed through the reservoir, then channel routed flow was subsequently combined with the runoff calculated for the Long Pond drainage area (Subarea 1) to form a composite hydrograph. The computed runoff hydrograph, Subarea 2, was then combined with the hydrograph from the two previous subareas and routed through Long Pond.

The all seasons 200 sq. miles 24 hours PMP for the Long Pond area, taken from Weather Bureau sources (Ref. 2) is 22 inches. The three unit hydrographs were computed by the Snyder method using coefficients of 2 and 0.6 for  $C_T$  and  $C_P$ , respectively. The in-flow hydrographs were developed by the U.S. Army Corps of Engineers HEC-1DB computer program (Ref. 1). Loss rates of 1.0 inch initial loss and 0.1 inch/hour constant loss were estimated as representative of the basin for the design storm.

In accordance with the recommended guidelines for Safety and Inspection of Dams (Ref. 3), the adequacy of the spillway was analyzed using the Probable Maximum Flood (PMF). A multiplan analysis was performed for the full 0.75, 0.50 and 0.25 PMF for two cases.

The Case I analysis considers discharge from the Long Pond reservoir during flood stage to be routed solely through the spillway located at the north abutment of the dam with the excess being permitted to overtop the structure.

Case II analysis considers the possibility that a portion of the highway embankment bounding the southeast end of the lake has a crest elevation approximately 1 ft lower than Long Pond dam and therefore would act as a secondary spillway in combination with the service spillway during a PMP event.

### 5.3 SPILLWAY CAPACITY

The service spillway located near the north abutment of Long Pond dam is a reinforced concrete box culvert 6.0 ft wide by 4.0 ft high, with an invert elevation estimated to be at 470 ft (MSL). The maximum computed discharge for a lake water surface at elevation 476 (top of dam) is 222 cfs.

The hypothesized secondary spillway provided by the low lying section of highway embankment at the southeast end of the lake has an estimated invert elevation of 475 (MSL) and an estimated trapezoidal section having a 250 ft base width with side slopes of approximately 0.5 percent. Routed flow would be over the road and routed north and south to two adjacent unnamed drainage areas. The computed maximum discharge of the "secondary spillway" for a lake level at 476 (MSL) is 669 cfs.

### 5.4 RESERVOIR CAPACITY

The normal reservoir capacity is listed as 115 acre-feet. The computed surcharge storage of 58.6 acre-feet is equivalent to approximately 2.4 inches of runoff over the entire basin.

### 5.5 FLOODS OF RECORD

There are no records available of Floods or maximum lake elevations.

### 5.6 OVERTOPPING POTENTIAL

The potential of the dam being overtopped was investigated on the basis of the spillway or spillways discharge capacities and the available surcharge storage to meet the selected design flood inflows for each of the two cases described previously.

The analyses were performed assuming that the water surface in the reservoir was at the service spillway crest elevation (El 470) at the start of the flood event. The computed PMF peak inflow was 1,647 cfs. The HEC-1DB analysis indicated that the dam would be overtopped by all floods exceeding 41 percent of the PMF for Case I. Table 1 is a summary of the computer analysis.

Table 1  
Results of Case I Analysis

<u>RATIO OF PMF</u>	<u>PEAK INFLOW (cfs)</u>	<u>PEAK OUTFLOW (cfs)</u>	<u>OVER- TOPPING (feet)</u>
1.00	1,647	1,630	1.14
0.75	1,224	1,214	0.90
0.50	806	761	0.60
0.25	242	159	0.00

For Case II the dam would be overtopped by all floods equal to an exceeding 58 percent of the PMF. The results of this analysis are presented in Table 2.

Table 2  
Results of Case II Analysis

<u>RATIO OF PMF</u>	<u>PEAK INFLOW (cfs)</u>	<u>PEAK OUTFLOW (cfs)</u>	<u>OVER- TOPPING (feet)</u>
1.00	1,647	1,628	0.22
0.75	1,224	1,222	0.11
0.50	806	847	0.00
0.25	242	159	0.00

#### 5.6 EVALUATION

The service spillway of the Long Pond dam is incapable of passing flows exceeding 41 percent of the PMF without the dam being overtopped. When considering the low roadway embankment at the southeastern corner of the reservoir (approximately 1,500 ft south of the dam) acting as a secondary spillway floods less than or equal to 50 percent of the PMF would not overtop the dam. Because of the uncertainty of the data regarding the "secondary spillway", the service spillway must be assessed as being seriously inadequate.

## SECTION 6 - STRUCTURAL STABILITY

### 6.1 EVALUATION OF STRUCTURAL STABILITY

#### a. Visual Observations

Visual observations did not reveal any conditions which at present adversely affect the structural stability of the dam. Seepage observed at the south abutment and flowing water, observed at the central toe of the dam did not show transportation of any soil particles and is therefore not considered an immediate hazard. The source of the flowing water at the toe, however, does require further investigation. Stability of the spillway structure appears adequate provided the downstream portions of the cracked and undercut slab are properly repaired. Location of the downstream reservoir drain discharge needs to be determined.

#### b. Design and Construction Drawings

There are no design or construction drawings in existence to the best knowledge of the owner.

#### c. Stability Analysis

Based on the results of visual observations, the stability of the dam appears adequate for a Phase I assessment. Similarly, stability of the service spillway is also judged as adequate.

#### d. Operating Records

There are no operating records for the dam.

#### e. Post-Construction Changes

There are reportedly no post-construction changes to the dam.

#### f. Seismicity Stability.

The dam is located in Seismic Zone 1 and in accordance with recommended Phase I guidelines. However, based on the past earthquake activity in the region, the New York Geological Survey considers the site to be more characteristic of a Zone 2 Setting. Based on this assessment, the dam is considered in the Seismic Zone 2.

## SECTION 7 - ASSESSMENT/RECOMMENDATIONS

### 7.1 ASSESSMENT

#### a. Safety

Examination of available documents and visual inspection of the dam and the appurtenant structures did not reveal conditions which constitute an immediate hazard to human life or property. However, the dam has some deficiencies which require further investigation and remedial action.

The earth embankment is considered to be stable under present operating conditions; however, poor condition of the downstream spillway slab, associated undercutting, and seepage observed at the south abutment may present a hazard condition under severe flood stage conditions. The stability of the dam is further endangered by the presence of a large masonry dam of unknown stability located immediately upstream of the reservoir.

Using the Corps of Engineers' Screening Criteria for review of spillway adequacy, it has been determined that the dam would be overtopped for all storms exceeding approximately 41 and 58 percent of the PMF for Cases I and II Analyses, as described in Section 5.2. The overtopping of the dam could cause the erosion of both abutments and the downstream face of the dam resulting in a breach, thus significantly increasing the hazard for loss of life downstream. The service spillway is therefore assessed as unsafe, non-emergency.

The classification of "unsafe" applied to a dam because of a "seriously inadequate" spillway is not meant to connote the same degree of emergency as would be associated with an "unsafe" classification applied for a structural deficiency. It does mean, however that based on an initial screening and preliminary computations, there appears to be a serious deficiency in spillway capacity so that if a severe storm were to occur, overtopping and failure of the dam would take place, significantly increasing the hazard to loss of life downstream from the dam.

b. Adequacy of Information

This report and its conclusions are based on visual inspection, interviews, and office hydrologic/hydraulic studies. This information and data are adequate for a Phase I inspection.

c. Need for Additional Investigations

Since the spillway is considered to be "seriously inadequate", additional hydrologic/hydraulic investigations are required to more accurately determine the site specific characteristics of the watershed including detailed information of the roadway embankment assumed to act as a secondary spillway for Case II analyses. After the in-depth hydrologic/hydraulic investigations have been completed, remedial measures must be initiated to provide spillway capacity sufficient to safely discharge the outflow from the  $\frac{1}{2}$  PMF event.

Additionally it is recommended that a dambreak type study be performed to assess the effect of the upstream North Lake reservoir on the safety of the Long Pond Dam.

d. Urgency

The additional hydrologic/hydraulic investigations which are required must be initiated within 3 months from the date of notification. Within 12 months of notification, remedial measures as a result of these investigations must be initiated, with completion of these measures during the following year. In the interim, an emergency action plan for the notification of downstream residents and proper governmental authorities in the event of overtopping should be developed, and around-the-clock surveillance of the dam during periods of extreme runoff should be provided. The other problem areas listed below must be corrected within one year from notification.

7.2 RECOMMENDED MEASURES

Recommended measures are as follows:

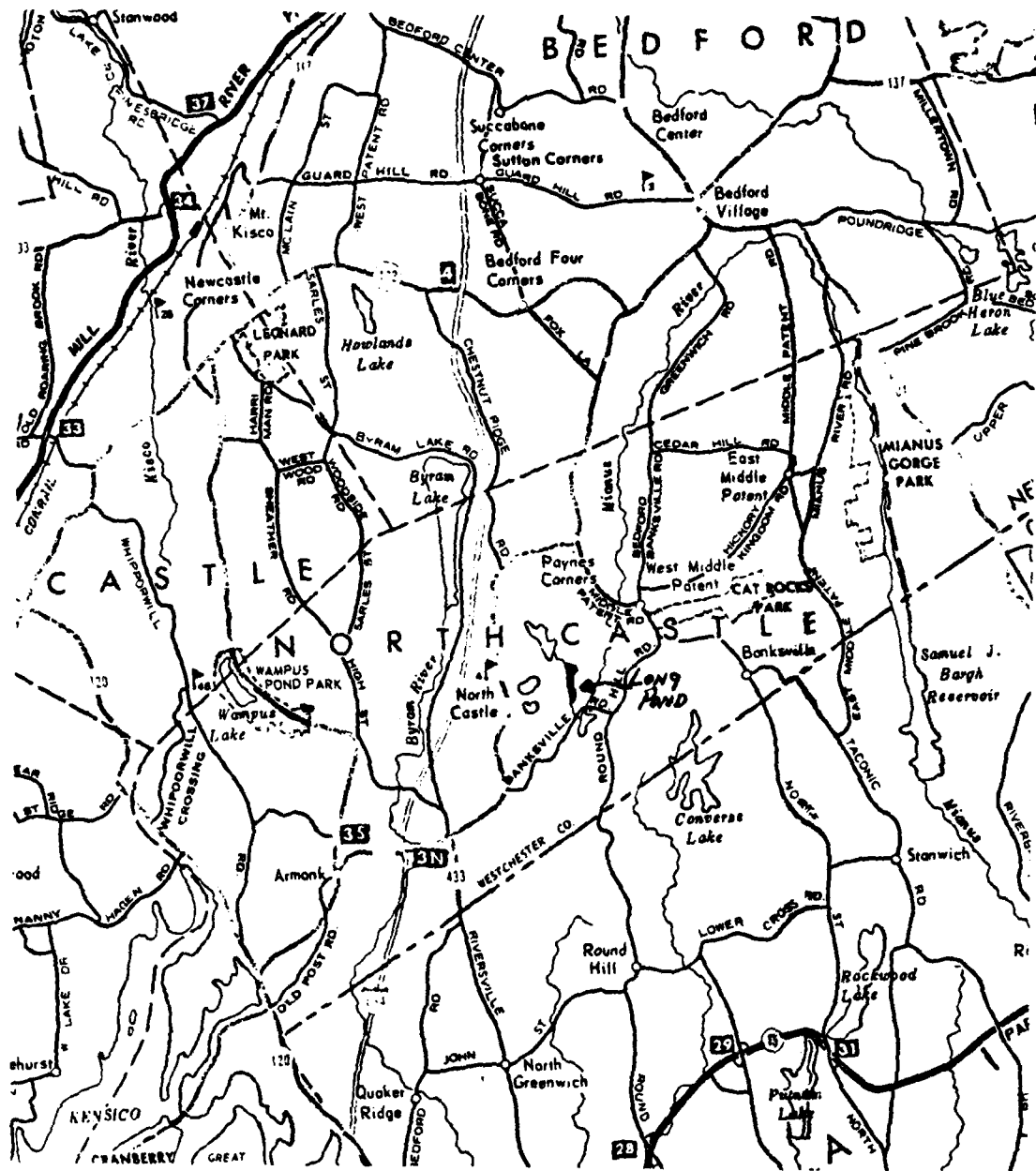
1. Nature of the seepage at the central toe of the dam should be investigated.



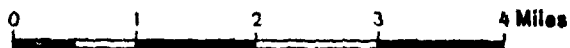
2. Location of the reservoir drain discharge should be determined.
3. Only after locating the reservoir drain discharge, the workability of the drain valve should be evaluated. If found to be inoperable or defective it should be repaired to a good working condition or replaced.
4. The concrete on the spillway training walls and downstream bottom slab should be repaired with proper backfilling of the unsupported portion of the slab being performed as part of this repair.
5. The rocklined spillway discharge channel should be evaluated to determine if it has sufficient capacity to contain calculated maximum spillway flows.
6. The seepage area at the south abutment should be blanketed with a properly filtered drainage blanket to allow controlled transportation of seepage waters downslope, in addition to minimizing the potential for piping of finer grained embankment soils.
7. All small trees, dead large trees and larger trees located near the crest should be cut down and removed. Larger trees on the downstream slope should be inventoried and their conditions and the area around the stump should be monitored for the development of seepage. Holes and depressions resulting from removal of trees should be backfilled and sealed.
8. The low lying area upstream of Duck Pond immediately adjacent to the toe of the dam should be backfilled to prohibit backwater encroachment on the downstream slope.
9. The upstream slope of the dam should be provided with proper slope protection against erosion.
10. A program of periodic inspection and maintenance of the dam and appurtenances including yearly operation and lubrication of the repaired gates should be provided. This program should be documented for future reference. The emergency action plan, described in Section 7.1d, should be maintained and updated periodically during the life of the structure.

DRAWINGS

APPENDIX A



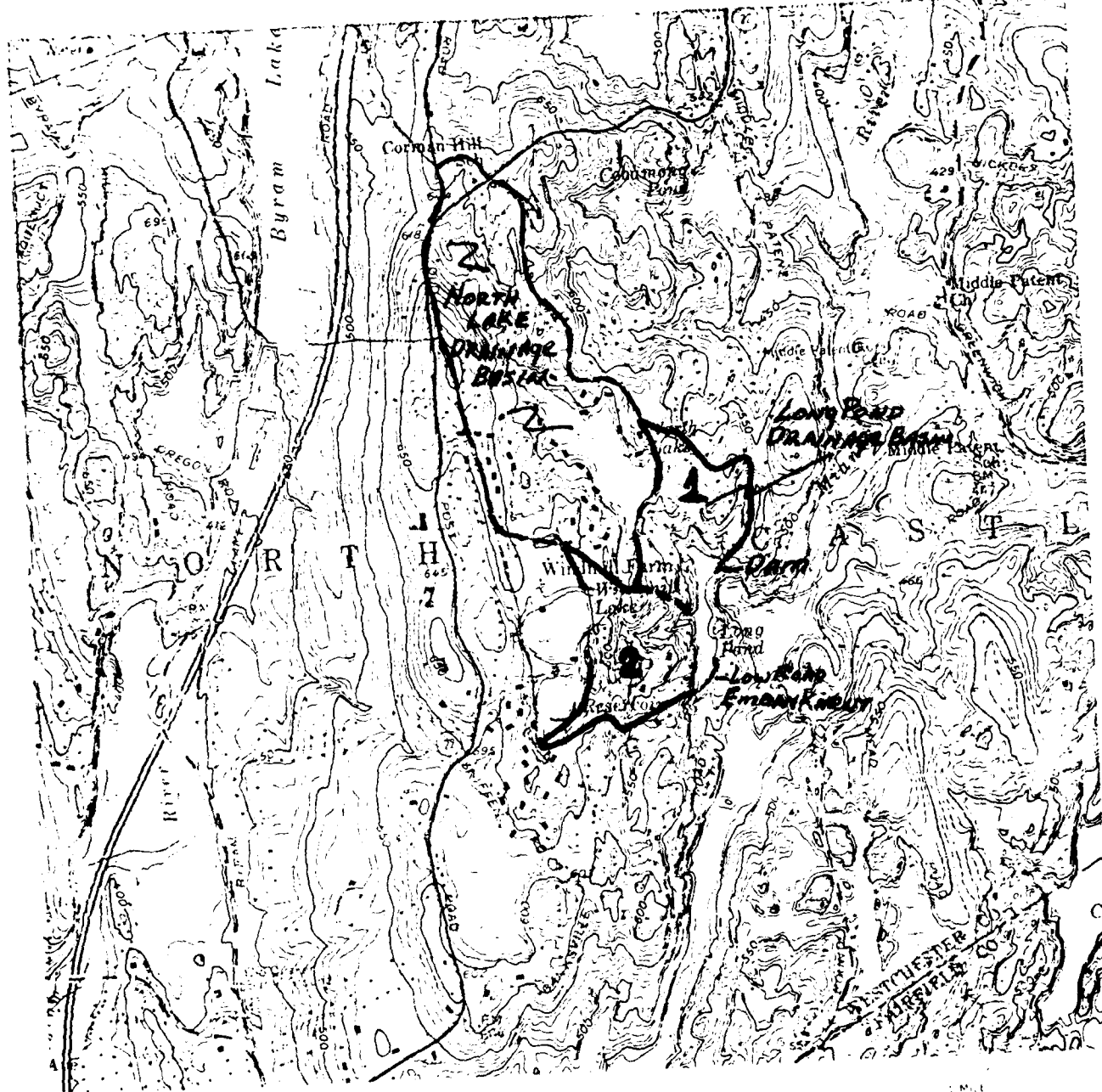
SCALE



VICINITY MAP

LONG POND DAM NO. 3

MT. KISCO, N.Y. QUAD



SCALE 1:24000

TOPOGRAPHIC MAP  
LONG POND DAM NO. 3

# TAMS

Job No. 1579-10

Sheet \_\_\_\_\_ of \_\_\_\_\_

Project Long Creek Dam No. 2

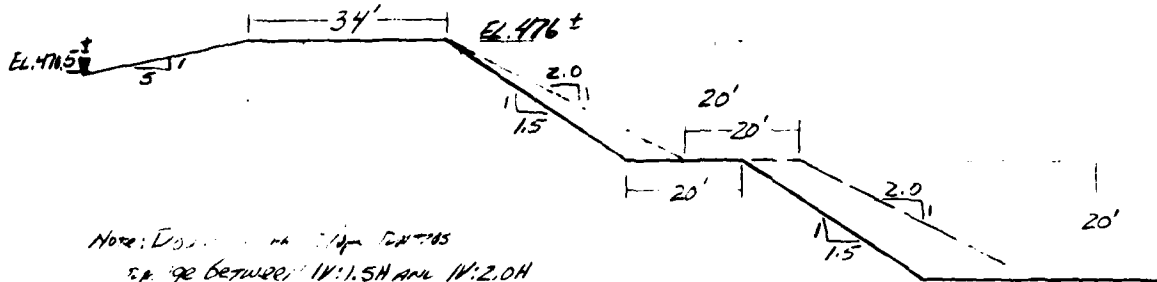
Date 6-26-81

Subject Dam, at 2nd Creek, near 1st

By JFW

CRST Alignment

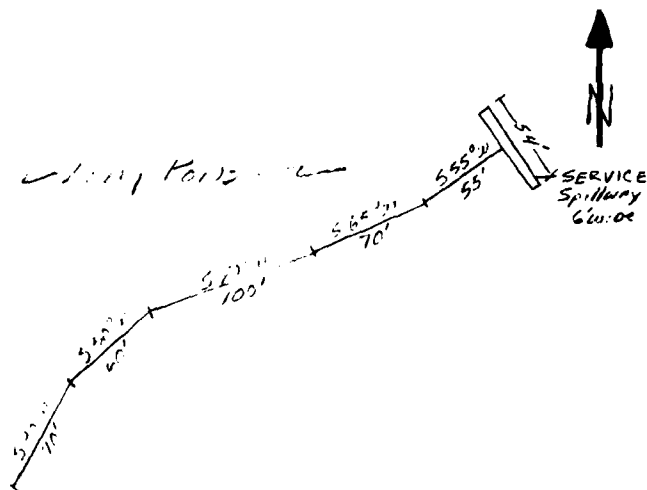
Ch'k. by \_\_\_\_\_



Note: Dams are 1/4" = 10' and 1/4" = 10' and 1/4" = 10'

Approximate Maximum Dam Section

Scale 1" = 30'



Approximate Alignment of Dam Structure

Scale 1" = 100'

PHOTOGRAPHS

APPENDIX B



2. VIEW FROM NORTH END OF CREST LOOKING  
SOUTH DOWN CENTER LINE OF DAM.



3. VIEW OF UPSTREAM SLOPE FROM NEAR  
SPILLWAY ENTRANCE.



4. VIEW OF DOWN-  
STREAM SLOPE  
AS SEEN FROM  
TOE. (NOTE:  
BENCHED SLOPE  
GEOMETRY)



5. VIEW OF DOWNSTREAM SLOPE AS SEEN FROM  
MID SLOPE.





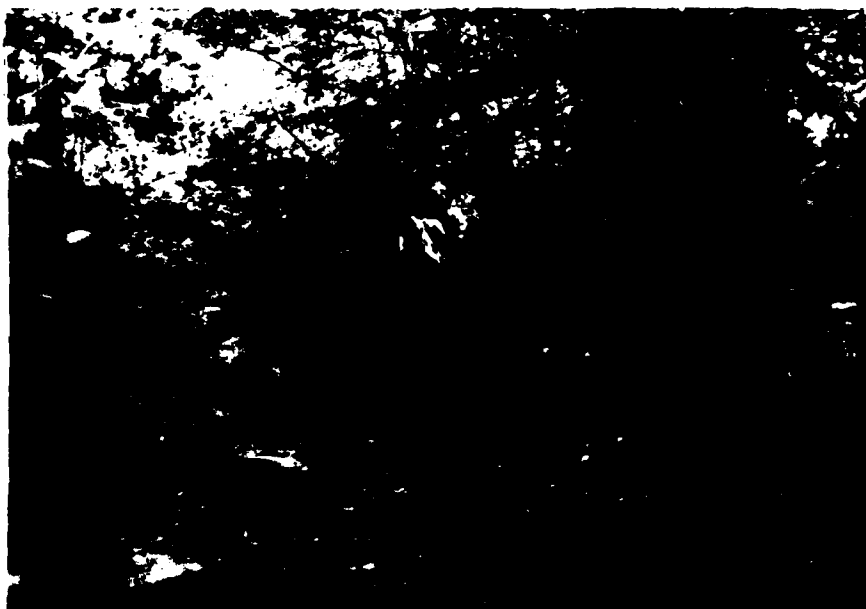
6. RESERVOIR DRAIN GATE HOUSE



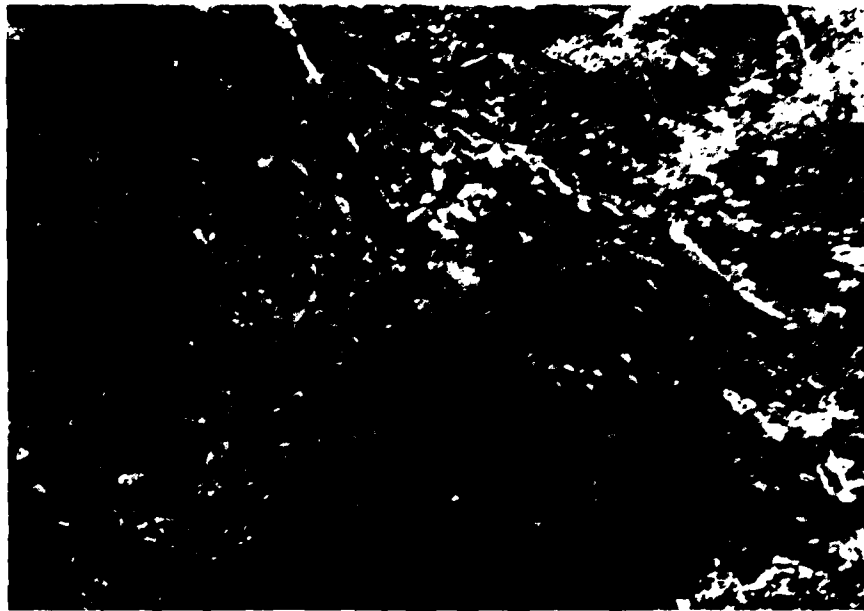
7. UPSTREAM VIEW OF SPILLWAY. (NOTE: SPALLING OF CONCRETE ON TRAINING WALLS).



8. CRACKED AND UNDERMINED SLAB AT DOWN-  
STREAM END OF SPILLWAY.



9. ROCK LINED SPILLWAY DISCHARGE CHANNEL,  
DOWNSTREAM NEAR TOE OF SLOPE.



10. OVERVIEW OF SEEPAGE AT CONTACT ON  
SOUTH ABUTMENT.



11. CLOSEUP VIEW  
OF SEEPAGE AREA  
REFERENCED  
ABOVE.



12. DISCHARGE AT CENTER TOE OF DOWNSTREAM SLOPE.  
PRESUMABLE BURIED RESERVOIR DRAIN OUTLET.



13. OVERVIEW OF LOW LYING DUCK POND BACKWATER  
AREA ADJACENT TO TOE OF DOWNSTREAM SLOPE.

VISUAL INSPECTION CHECKLIST

APPENDIX C

# VISUAL INSPECTION CHECKLIST

## 1) Basic Data

### a. General

Name of Dam Long Pond Dam No. 3  
Fed. I.D. # NY00115 DEC Dam No. 1  
River Basin Mianus River  
Location: Town North Castle County Westchester  
Stream Name UNNAMED DRAINAGE  
Tributary of Mianus River  
Latitude (N) 41-08.5 Longitude (W) 073-40.4  
Type of Dam EARTH AND ROCKFILL  
Hazard Category HIGH (1)  
Date(s) of Inspection 9 JUNE 1981  
Weather Conditions OVCYCAST & INTERMITTENT RAIN  
Reservoir Level at Time of Inspection 270.5' (ESTIMATED BASED ON USGS MAP)

b. Inspection Personnel HARVEY FELDMAN - PRINCIPAL GEOTECHNICAL ENG.  
JOHN F. WALLACE - GEOTECHNICAL ENGINEER

c. Persons Contacted (Including Address & Phone No.) MR. K. KARL MUELLER  
LONG POND CT., (WINOMILL FARMS, ARMONK, N.Y. 10504  
(914)-273-8074

### d. History:

Date Constructed UNKNOWN (CIRCA 1932) Date(s) Reconstructed —

Designer ELWYNE E. SEELEY & CO., CONSULTING ENGINEERS N.Y., N.Y.  
Constructed By UNKNOWN  
Owner MR. K. KARL MUELLER, LONG POND CT. ARMONK, N.Y.

2 Embankment

a. Characteristics

- (1) Embankment Material EARTH AND ROCK FILL
- (2) Cutoff Type NONE KNOWN TO EXIST
- (3) Impervious Core NONE KNOWN TO EXIST
- (4) Internal Drainage System NONE KNOWN TO EXIST
- (5) Miscellaneous \_\_\_\_\_

b. Crest

- (1) Vertical Alignment GOOD
- (2) Horizontal Alignment GOOD
- (3) Surface Cracks NONE
- (4) Miscellaneous \_\_\_\_\_

c. Upstream Slope

- (1) Slope (Estimate) (V:H) 1V:5H to 1V:6H
- (2) Undesirable Growth or Debris, Animal Burrows NUMEROUS bushes  
OCCASIONAL small (smaller than 8" diam) deciduous trees
- (3) Sloughing, Subsidence or Depressions NONE observed

(4) Slope Protection NONE

(5) Surface Cracks or Movement at Toe NONE observed

d. Downstream Slope

(1) Slope (Estimate - V:H) Broken Slope 1V:1.5H to 1V:2.0H w/ 20' bench 20' below crest  
Here North Abutment spillway channel depth is 1V:1.3H

(2) Undesirable Growth or Debris, Animal Burrows NUMEROUS MICE

As (8" to 18" diameter) deciduous trees - bushes & shrubs at crest

(3) Sloughing, Subsidence or Depressions NONE observed

(4) Surface Cracks or Movement at Toe NONE

(5) Seepage seepage at mid slope of south abutment - seepage flow estimated to be 5 to 7 gpm. no soil erosion

(6) External Drainage System (Ditches, Trenches; Blanket) spillway channel located on downstream slope none earth abutment.

(7) Condition Around Outlet Structure some minor cutting of spillway slab

(8) Seepage Beyond Toe NONE observed

e. Abutments - Embankment Contact

seepage at contact of south abutment with adjacent slope



(1) Erosion at Contact NONE

(2) Seepage Along Contact AT MIDPOINT OF WEST ABUTMENT -

QUANTITY EST BE BE 5 to 7 gpm

3) Drainage System

a. Description of System NONE

b. Condition of System N/A

c. Discharge from Drainage System N/A

Instrumentation (Monumentation/Surveys, Observation Wells, Weirs,  
Piezometers, Etc.) NONE OBSERVED

Reservoir

- a. Slopes Appear to be generally stable. no sign of  
sloughing or instability in vicinity of Dam or south shoreline
- b. Sedimentation NONE
- c. Unusual Conditions Which Affect Dam LOCATION OF NORTH LAKE DAM  
IMMEDIATELY HOSTILE

6. Area Downstream of Dam

- a. Downstream Hazard (No. of Homes, Highways, etc.) SEVERAL RESIDENTIAL  
HOUSES -
- b. Seepage, Unusual Growth NONE
- c. Evidence of Movement Beyond Toe of Dam NONE
- d. Condition of Downstream Channel NATURAL SWAIL WITH FEATHERS  
OTHERWISE GOOD

7. Spillway(s) (Including Discharge Conveyance Channel)

- a. General RECTANGULAR REINFORCED BOX CULVERT LOCATED  
AT NORTH END OF DAM - 4' high by 6' wide SECTION 54' in  
length including approach and discharge SLABS - Walls are  
1 ft thick with
- b. Condition of Service Spillway SOME SPALLING OF TRAINING WALLS AND  
APPROACH EDGES - BOTTOM SLAB ON DOWNSTREAM END IS  
CRACKED AND PARTIALLY SEPARATED APPROXIMATELY 3-5 feet  
FROM END. SOME UNDER CUTTING OF SLAB SUPPORT  
HAS RESULTED

- c. Condition of Auxiliary Spillway Roadway Embankment located on  
south end of lake may have been constructed on two feet  
lower than the embankment and the latter could function  
as a secondary spillway under peak flood (PMF) conditions  
Present Condition is good - most supports are highly tapered and
- d. Condition of Discharge Conveyance Channel. Channel Section on  
Downstream Dam Face, heavily strewn with large massive  
boulders - no significant erosion - Section beyond toe  
in natural channel - relatively steep except for occasional  
deciduous trees - good condition

8) Reservoir Drain/Outlet

Type: Pipe UNKNOWN Conduit        Other       

Material: Concrete UNKNOWN Metal        Other       

Size: UNKNOWN Length UNKNOWN

Invert Elevations: Entrance UNKNOWN Exit UNKNOWN

Physical Condition (Describe):        Unobservable X

Material: UNKNOWN

Joints:        Alignment       

Structural Integrity: UNKNOWN

Hydraulic Capability: UNKNOWN

Means of Control: Gate ? Valve ? Uncontrolled       

Operation: Operable        Inoperable        Other UNKNOWN

Present Condition (Describe): Outlet downstream of dam is presumably  
buried under several feet of boulders - gate house is situated in Reservoir  
and was inaccessible

- a. Concrete Surfaces SEE ITEM 7 FOLLOWING
- b. Structural Cracking SEE ITEM 76
- c. Movement - Horizontal & Vertical Alignment (Settlement) SEE ITEM 76
- d. Junctions with Abutments or Embankments SPILLWAY JUNCTIONS WITH DAM ARE GOOD WITH THE EXCEPTION OF THE DOWNSTREAM TO THE BOTTOM SLAB - UNDERSTANDING HAS CAUSED DISCREPANCY TO OCCUR
- e. Drains - Foundation, Joint, Face NONE OBSERVED
- f. Water Passages, Conduits, Sluices SEE ITEM 7
- g. Seepage or Leakage LEAKAGE BELOW DOWNSTREAM SPILLWAY CHANNEL SLAB THROUGH GATE AND SEEPAGE LOCATED 46.6 FEET UPSTREAM OF DOWNSTREAM GATE

- h. Joints - Construction, etc. None observed
- i. Foundation
- j. Abutments
- k. Control Gates NONE
- l. Approach & Outlet Channels See Item 7B
- m. Energy Dissipators (Plunge Pool, etc.) NONE
- n. Intake Structures NOT observed
- o. Stability GOOD
- p. Miscellaneous

10) Appurtenant Structures (Powerhouse, Lock, Gatehouse, Other)

a. Description and Condition

see item 7 Spillway

NO OTHER APPURTENANT STRUCTURES  
WERE OBSERVED

HYDROLOGIC DATA AND COMPUTATIONS

APPENDIX D

# LONG POND DAM #3.

## CHECK LIST FOR DAMS HYDROLOGIC AND HYDRAULIC ENGINEERING DATA

1

### AREA-CAPACITY DATA:

	<u>Elevation*</u> (ft.)	<u>Surface Area</u> (acres)	<u>Storage Capacity</u> (acre-ft.)
1) Top of Dam	<u>476</u>	<u>11.28</u>	<u>173.6</u>
2) Design High Water (Max. Design Pool)	<u>Unknown</u>	<u>      </u>	<u>      </u>
3) Auxiliary Spillway Crest	<u>475<sup>+</sup></u> <u>Low highway embankment</u>	<u>      </u>	<u>      </u>
4) Pool Level with Flashboards	<u>      </u>	<u>      </u>	<u>      </u>
5) Service Spillway Crest	<u>470</u>	<u>8.26</u>	<u>115</u>

### DISCHARGES

	<u>Volume</u> (cfs)
1) Average Daily	<u>Unknown</u>
2) Spillway @ Maximum High Water	<u>222</u>
3) Spillway @ Design High Water	<u>Unknown</u>
4) Spillway @ Auxiliary Spillway Crest Elevation	<u>Unknown</u>
5) Low Level Outlet	<u>Unknown</u>
6) Total (of all facilities) @ Maximum High Water	<u>222<sup>+</sup></u>
7) Maximum Known Flood	<u>Unknown</u>
8) At Time of Inspection	<u>Unknown</u>

\* All ELEVATIONS ARE BASED ON POND LEVEL DATUM  
RELATIVE TO AN ELEVATION INTERPOLATED FROM THE USGS NORTON LAKE, NY.  
QUAD Sect.



Embarked  
CREST:

ELEVATION: 476

Type: Exposed Rock Fill

Width: 34'

Length: 365'

Spillover NONE

Location                     

SPILLWAY:

SERVICE

AUXILIARY

470'

INVERT

Elevation

4'0" H X 6'0" W box culvert

Type

Approx 54.0' in length

Width

Type of Control

☒ Uncontrolled

Controlled:

Type  
(Flashboards; gate)

Number

Size/Length

Invert Material

Anticipated Length  
of operating service

Chute Length

Height Between Spillway Crest  
& Approach Channel Invert  
(Weir Flow)

HYDROMETEOROLOGICAL GAGES:

NONE USED

Type : \_\_\_\_\_

Location: \_\_\_\_\_

Records:

Date - \_\_\_\_\_

Max. Reading - \_\_\_\_\_

FLOOD WATER CONTROL SYSTEM:

Warning System: \_\_\_\_\_

NONE

Method of Controlled Releases (mechanisms):

gate valve on Reservoir Dam

DRAINAGE AREA: \_\_\_\_\_

0.52 SQ MI

## DRAINAGE BASIN RUNOFF CHARACTERISTICS:

Land Use - Type: \_\_\_\_\_

WOODS - SMALL FARMS

Terrain - Relief: \_\_\_\_\_

Hilly

Surface - Soil: \_\_\_\_\_

GLACIAL TILL

Runoff Potential (existing or planned extensive alterations to existing  
(surface or subsurface conditions)

None observed

Potential Sedimentation problem areas (natural or man-made; present or future)

None

Potential Backwater problem areas for levels at maximum storage capacity  
including surcharge storage:

None

Dikes - Floodwalls (overflow & non-overflow) - Low reaches along the  
Reservoir perimeter:

Location: \_\_\_\_\_

Elevation: \_\_\_\_\_

Reservoir:

Length @ Maximum Pool \_\_\_\_\_

0.096 (NOCAL to Dam Alvarado) (Miles)

Length of Shoreline @ Spillway Crest) 0.14 mi. \_\_\_\_\_

(Miles)

# TAMS

Job No. 1579-10  
 Project LONG POND DAM No 3  
 Subject HYDROLOGIC/HYDRAULIC COMPUTATIONS  
HYDROLOGIC UNIT CODE NO 01100006

Sheet 1 of 85  
 Date JUNE 18, 21  
 By LEE  
 Ch'k. by \_\_\_\_\_

## ASSUME:

1. BASIN UPSTREAM OF WINDMILL LAKE IS COMPLETELY CONTROLLED & WILL NOT CONTRIBUTE TO LONG POND INFLOW
  2. FOR ANALYSIS BASIN IS DIVIDED INTO 3 SUB-AREAS
    - (1) North Lake sub-area.
    - (2) Sub-area 1 downstream of North Lake.
    - (3) Sub-area 2 West of Long Pond extending to Windmill Lake.
- TOTAL AREA = 0.46 SQUARE MILES.

SUB-AREA 1 (59.2 ac = 0.09 mi<sup>2</sup>)

$$L = 1900' = 0.34 \text{ miles}$$

$$L_A = 400' = 0.076 \text{ miles}$$

$$\text{Use } C_T = 2 \text{ \& } 640 C_P = 400 \quad C_P = 0.625$$

$$t_P = 2 (0.34 \times 0.076)^{0.75} = 0.68 \text{ hrs}$$

$$t_A = 0.001 \left( \frac{400}{59.2} \right) = 0.12 \text{ hrs}$$

$$\text{For } t_R = 0.33 \text{ hrs}$$

$$t_{PR} = t_P + 0.25(0.33 - 0.12) = 0.73 \text{ hours}$$

$$\% \text{ Impervious } 4.13/59.2 = 0.07$$

SUB-AREA 2 (67.5 ac = 0.11 mi<sup>2</sup>)

$$L = 1600' = 0.3 \text{ miles}$$

$$L_A = 600' = 0.11 \text{ miles}$$

$$\text{Use } C_T = 2 \text{ \& } 640 C_P = 400$$

$$t_P = 2 (0.3 \times 0.11)^{0.75} = 0.73 \text{ hrs}$$

$$t_A = 0.001 \left( \frac{400}{67.5} \right) = 0.13 \text{ hrs}$$

# TAMS

Job No. 1579-10

Sheet 2 of 35

Project LONG POND DAM #3

Date Tu 11/1/01

Subject HYDROLOGIC/HYDRAULIC COMPUTATIONS

By DLS

Ch'k. by \_\_\_\_\_

SUB AREA 2 CON- %impervious  $\frac{4.13}{67.5} = 0.06$

$$t_{PR} = t_P + 0.25(t_R - t_R)$$

For  $t_R = 0.33$  hrs.

$$t_{PR} = 0.72 + 0.25(0.33 - 0.13) = 0.77 \text{ hours}$$

NORTH LAKE Sub-basin. (from North Lake DAM #3)  
(PHASE 1 REPORT)

$$t_{PR} = 0.98 \text{ hrs}$$

$$t_R = 0.33 \text{ hrs}$$

# TAMS

Job No. 1579-10

Sheet 3 of 85

Project LONG POND DAM No 3

Date JUNE 13, 81

Subject HYDROLOGIC / HYDRAULIC Computations

By D.L.C.

Ch'k. by J.M.D.

## SPILLWAY RATING INVERT EL 470.

USE MANNING FORMULA for depth (d) = 1, 2, 3, 5.

$Q = \frac{1.49}{n} A R^{2/3} S^{1/2}$						
d	$\frac{1.49}{n}$	A	P	A/P	$R^{2/3}$	$S^{1/2}$
1	99.3	6	8	.75	.65	24.6
2		12	10	1.2	.85	67.3
3.5		21	13	1.6	1.5	142.6

S = 0.0025

n = 0.015

$H/d$	H	$Q/b$	$Q_1$	(See attached chart, Open-Channel Hydraulics - Chow)	
0.3	1.2	4	24	471.2	
0.5	2.0	8.8	53	472	
1.0	4	25	150	474	Top of low subject.
1.25	5	30	160	475	
1.375	5.5	33.5	201	475.5	
1.5	6	37	222	476	TOP OF DAM EL
2.0	8	45	270	478	
2.5	10	53	318	480	

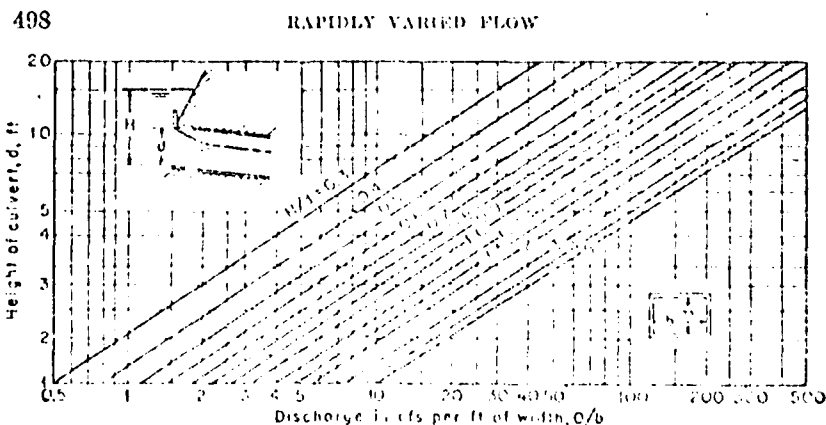


FIG. 17-29 Chart for estimating headwater on box culverts with square edged entrances, flowing partly full. (Based on data of U.S. Bureau of Public Roads [20].)

# TAMS

Joh No. 1579-15

Sheet 4 of 35

Project Levee Pond Dam No. 3

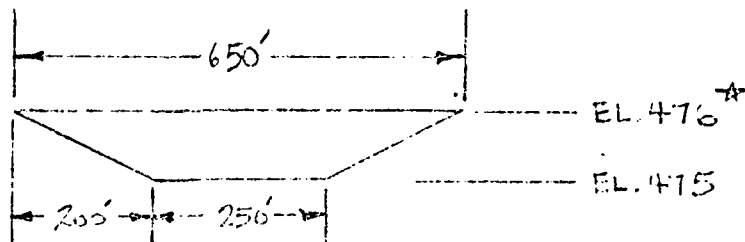
Date 6/25/81

Subject LOW SPOT IN RELEVANT WHICH IS TO BE

By H. O. D. C.

SECONDARY SPILLWAY - DISCHARGE VS. ELEVATION

Ch'k. by \_\_\_\_\_



ASSUMED:  $n = 0.025$  (LEVEE & ROADWAY SURFACES)  
 $S = 0.001 \Rightarrow S^K = 0.002$

ELEV. (EL)	A ( $G^2$ )	P ( $G^3$ )	R ( $G^4$ )	$P^2$ ( $G^5$ )	$Q = \frac{1.49}{n} AR^{2/3} S^{1/2} = 1.9072 AR^{2/3}$
475					0
475.5	175	450	0.39	0.53	177
476	450	650	0.69	0.78	669
478	1750	654	2.68	1.93	6523

\* ABOVE EL. 476, THE SPILLWAY WIDTH WILL REMAIN AT 650 FT.

NORTH LAKE (104115) - STRENGTH:

ELEVATION	AM	AREA AC	PERIMETER AC	A VOL AC-FT	STORAGE AC-FT
572		21.6			210
574	2	22.9	22.25	44.5	222.5
575	1	23.6	23.75	23.65	277.75
577	2	25.0	24.8	49.6	327.25
580	3	27.1	26.95	81.15	408.5

# TAMS

Job No. 1572-10

Sheet 5 of 25

Project Long Pond Dam

Date July 1971

Subject Hydrologic Investigation

By D.L.C.

Ch'k. by \_\_\_\_\_

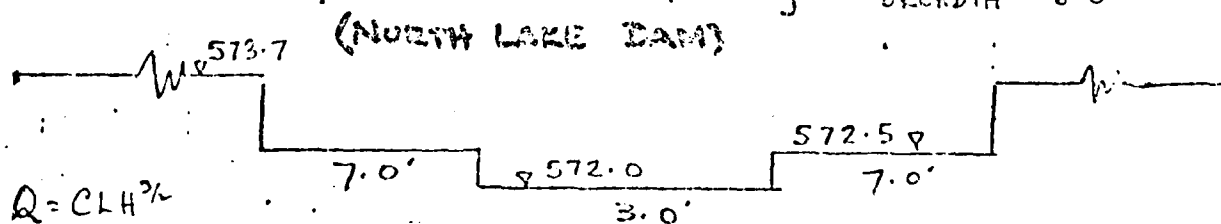
## LONG POND - SPILLWAY

(SE Corner of Lake)

DISCHARGE	Q <sub>1</sub>	FLOW OVER LOW SPOT	TOTAL
	(PRIMARY SPILLWAY)	Q <sub>2</sub>	Q <sub>T</sub>
470	0	0	0
471.2	24	0	24
472.	53	0	53
474	150	0	150
475	180	0	180
475.5	201	177	378
476	222	669	891
478	270	6432	6702
474			475

## SPILLWAY DISCHARGE CAPACITY

BREADTH ≈ 60'



$Q = CLH^{3/2}$

EL.	L <sub>1</sub>	H <sub>1</sub>	C <sub>1</sub>	Q <sub>1</sub> (CFS)	L <sub>2</sub>	H <sub>2</sub>	C <sub>2</sub>	Q <sub>2</sub>	Q <sub>T</sub>
572	3	0	0		0	0		0	
573	3	1	2.68	8	14	0.5	2.6	12.9	20.9
573.75	3	1.75	2.65	18.4	14	1.25	2.66	52.0	70.4
575	3	3	2.66	41.5	14	2.5	2.67	147.8	189.3



# TAMS

Job No. 1579-10

Sheet 6 of 8

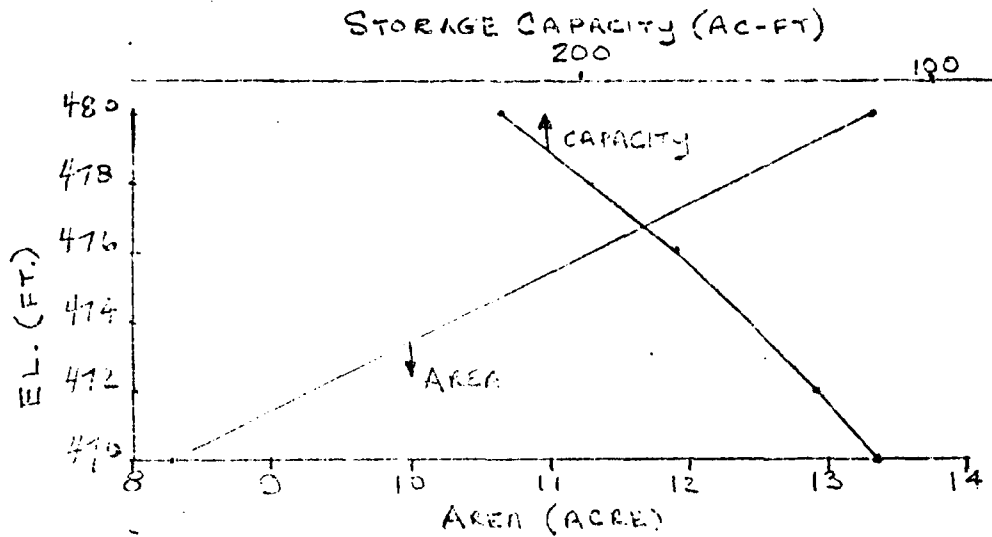
Project LONG POND DAM No 3

Date JUNE 18

Subject HYDROLOGIC / HYDRAULIC COMPUTATIONS

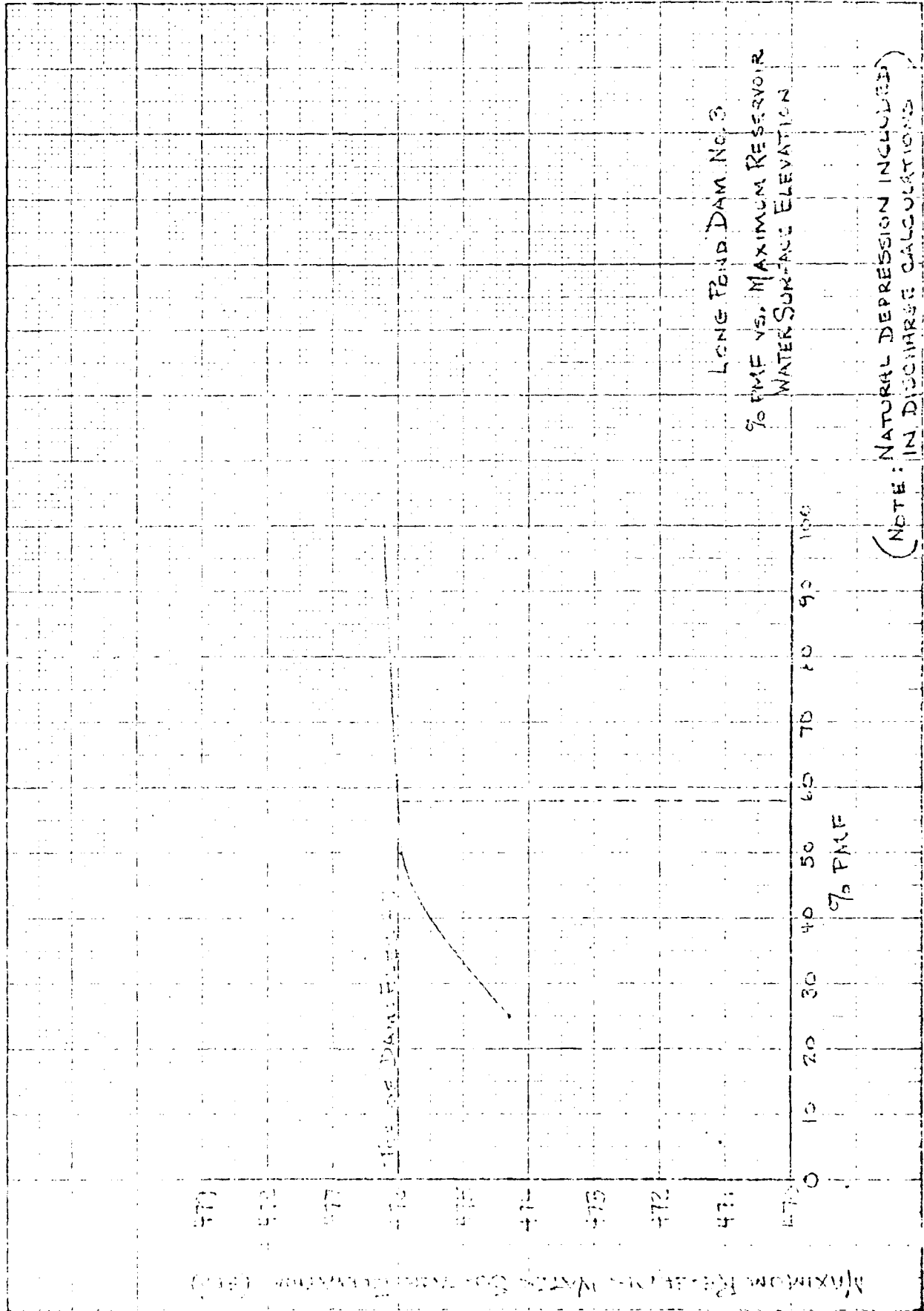
By D.E.C.

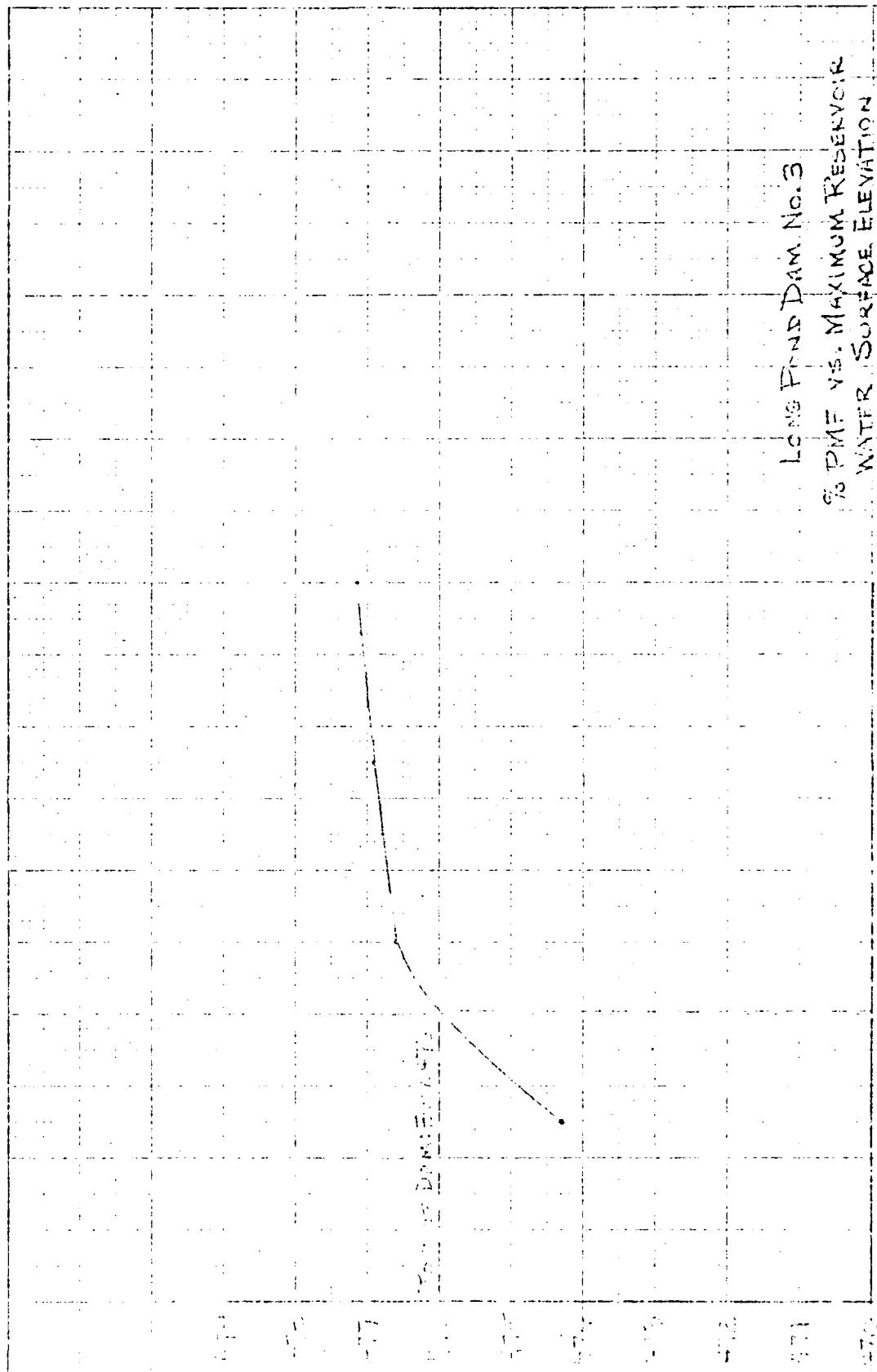
Ch'k. by \_\_\_\_\_



EL.	ΔH	AREA	MEAN AREA	Δ Vol	STORAGE
470		8.26			115*
472	2	9.27	8.765	17.53	132.5
474	2	10.28	9.775	19.55	152.1
476	2	11.25	10.78	21.56	173.6
480	4	13.3	12.29	49.16	222.8

124





LONG POND DAM NO. 3

% PMF VS. MAXIMUM RESERVOIR

WATER SURFACE ELEVATION

NO. 107

(NOTE: NATURAL DEPRESSION NOT INCLUDED)



# 10. ROUTE THROUGH LONG POND

ST	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46	47	48	49	50	51	52	53	54	55	56	57	58	59	60	61	62	63	64	65	66	67	68	69	70	71	72	73	74	75	76	77	78	79	80	81	82	83	84	85	86	87	88	89	90	91	92	93	94	95	96	97	98	99	100	101	102	103	104	105	106	107	108	109	110	111	112	113	114	115	116	117	118	119	120	121	122	123	124	125	126	127	128	129	130	131	132	133	134	135	136	137	138	139	140	141	142	143	144	145	146	147	148	149	150	151	152	153	154	155	156	157	158	159	160	161	162	163	164	165	166	167	168	169	170	171	172	173	174	175	176	177	178	179	180	181	182	183	184	185	186	187	188	189	190	191	192	193	194	195	196	197	198	199	200	201	202	203	204	205	206	207	208	209	210	211	212	213	214	215	216	217	218	219	220	221	222	223	224	225	226	227	228	229	230	231	232	233	234	235	236	237	238	239	240	241	242	243	244	245	246	247	248	249	250	251	252	253	254	255	256	257	258	259	260	261	262	263	264	265	266	267	268	269	270	271	272	273	274	275	276	277	278	279	280	281	282	283	284	285	286	287	288	289	290	291	292	293	294	295	296	297	298	299	300	301	302	303	304	305	306	307	308	309	310	311	312	313	314	315	316	317	318	319	320	321	322	323	324	325	326	327	328	329	330	331	332	333	334	335	336	337	338	339	340	341	342	343	344	345	346	347	348	349	350	351	352	353	354	355	356	357	358	359	360	361	362	363	364	365	366	367	368	369	370	371	372	373	374	375	376	377	378	379	380	381	382	383	384	385	386	387	388	389	390	391	392	393	394	395	396	397	398	399	400	401	402	403	404	405	406	407	408	409	410	411	412	413	414	415	416	417	418	419	420	421	422	423	424	425	426	427	428	429	430	431	432	433	434	435	436	437	438	439	440	441	442	443	444	445	446	447	448	449	450	451	452	453	454	455	456	457	458	459	460	461	462	463	464	465	466	467	468	469	470	471	472	473	474	475	476	477	478	479	480	481	482	483	484	485	486	487	488	489	490	491	492	493	494	495	496	497	498	499	500	501	502	503	504	505	506	507	508	509	510	511	512	513	514	515	516	517	518	519	520	521	522	523	524	525	526	527	528	529	530	531	532	533	534	535	536	537	538	539	540	541	542	543	544	545	546	547	548	549	550	551	552	553	554	555	556	557	558	559	560	561	562	563	564	565	566	567	568	569	570	571	572	573	574	575	576	577	578	579	580	581	582	583	584	585	586	587	588	589	590	591	592	593	594	595	596	597	598	599	600	601	602	603	604	605	606	607	608	609	610	611	612	613	614	615	616	617	618	619	620	621	622	623	624	625	626	627	628	629	630	631	632	633	634	635	636	637	638	639	640	641	642	643	644	645	646	647	648	649	650	651	652	653	654	655	656	657	658	659	660	661	662	663	664	665	666	667	668	669	670	671	672	673	674	675	676	677	678	679	680	681	682	683	684	685	686	687	688	689	690	691	692	693	694	695	696	697	698	699	700	701	702	703	704	705	706	707	708	709	710	711	712	713	714	715	716	717	718	719	720	721	722	723	724	725	726	727	728	729	730	731	732	733	734	735	736	737	738	739	740	741	742	743	744	745	746	747	748	749	750	751	752	753	754	755	756	757	758	759	760	761	762	763	764	765	766	767	768	769	770	771	772	773	774	775	776	777	778	779	780	781	782	783	784	785	786	787	788	789	790	791	792	793	794	795	796	797	798	799	800	801	802	803	804	805	806	807	808	809	810	811	812	813	814	815	816	817	818	819	820	821	822	823	824	825	826	827	828	829	830	831	832	833	834	835	836	837	838	839	840	841	842	843	844	845	846	847	848	849	850	851	852	853	854	855	856	857	858	859	860	861	862	863	864	865	866	867	868	869	870	871	872	873	874	875	876	877	878	879	880	881	882	883	884	885	886	887	888	889	890	891	892	893	894	895	896	897	898	899	900	901	902	903	904	905	906	907	908	909	910	911	912	913	914	915	916	917	918	919	920	921	922	923	924	925	926	927	928	929	930	931	932	933	934	935	936	937	938	939	940	941	942	943	944	945	946	947	948	949	950	951	952	953	954	955	956	957	958	959	960	961	962	963	964	965	966	967	968	969	970	971	972	973	974	975	976	977	978	979	980	981	982	983	984	985	986	987	988	989	990	991	992	993	994	995	996	997	998	999	1000	1001	1002	1003	1004	1005	1006	1007	1008	1009	1010	1011	1012	1013	1014	1015	1016	1017	1018	1019	1020	1021	1022	1023	1024	1025	1026	1027	1028	1029	1030	1031	1032	1033	1034	1035	1036	1037	1038	1039	1040	1041	1042	1043	1044	1045	1046	1047	1048	1049	1050	1051	1052	1053	1054	1055	1056	1057	1058	1059	1060	1061	1062	1063	1064	1065	1066	1067	1068	1069	1070	1071	1072	1073	1074	1075	1076	1077	1078	1079	1080	1081	1082	1083	1084	1085	1086	1087	1088	1089	1090	1091	1092	1093	1094	1095	1096	1097	1098	1099	1100	1101	1102	1103	1104	1105	1106	1107	1108	1109	1110	1111	1112	1113	1114	1115	1116	1117	1118	1119	1120	1121	1122	1123	1124	1125	1126	1127	1128	1129	1130	1131	1132	1133	1134	1135	1136	1137	1138	1139	1140	1141	1142	1143	1144	1145	1146	1147	1148	1149	1150	1151	1152	1153	1154	1155	1156	1157	1158	1159	1160	1161	1162	1163	1164	1165	1166	1167	1168	1169	1170	1171	1172	1173	1174	1175	1176	1177	1178	1179	1180	1181	1182	1183	1184	1185	1186	1187	1188	1189	1190	1191	1192	1193	1194	1195	1196	1197	1198	1199	1200	1201	1202	1203	1204	1205	1206	1207	1208	1209	1210	1211	1212	1213	1214	1215	1216	1217	1218	1219	1220	1221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# THE VIEW OF SEQUENCE OF STREAM NETWORK CALCULATIONS

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 FLOOD HYDROGRAPH PROGRAM (FHC-1)  
 LAW SAFETY VERSION JULY 1976  
 LAST MODIFICATION 14 APR 76  
 .....

POL DATE 11/07/76  
 TIME 11:13:24

LONG POND DAM NO 3  
 PHASE 1 SAFETY INSPECTION  
 FHC FOR PMF ANALYSIS

JOB SPECIFICATION  
 NO SUP AWIN ICAY IPR IMIN METRC JPLT JPRT NSTAN  
 150 0 20 0 0 0 0 0 0 0 0  
 JUPES NWI LPOST TRACE  
 0 0 0 0

MULTI-PLAN ANALYSES TO BE PERFORMED  
 NPLAN= 1 NRTO= 4 LRTO= 1  
 STAGE 1.07 .75 .50 .25

SUB-AREA PUNDFF COMPUTATION

1 NORTH LANE BASIN RUN-OFF

ISTAG ICOMP IECN ITHA JPLT JPRT ISNAME ISTAGE IAUTO  
 1 0 0 0 0 0 1 0 0

HYDROGRAPH DATA

INQSG IUNG TAREA SAPP TPSTA TSPC PATIO ISNOW ISARE LOCAL  
 1 1 .26 0.00 .66 0.00 0.00 0 1 0  
 PRECIP DATA  
 SPSF PMS PA P12 P24 P40 R72 P96  
 0.00 22.00 112.00 123.00 133.00 141.00 0.00 0.00  
 TABLE COMPUTED BY THE PROGRAM IS .F03

LOSS DATA

LEOPT SIKPR OLKE RTIOU ERAIN STHAC RTIOX STREL CNEL ALSMY PTIMP  
 0 0.00 0.00 1.00 0.00 0.00 1.00 .10 0.00 .13

UNIT HYDROGRAPH DATA  
 TPE .9% CP= .63 NTA= 0

RECESSION DATA

STAGE= 1.00 CRUSSE= .10 RTIORE= 1.50  
 APPROXIMATE CLARK COEFFICIENTS FROM GIVEN SNYDER CP AND TP ARE TCF= 1.63 AND R= 2.56 INTERVALS

UNIT HYDROGRAPH IS END-OF-PERIOD ORDINATES, LAG= .97 HOURS, CP= .62 VOL= 1.00  
 16. 62. 101. 103. 76. 50. 32. 21. 14. 9.  
 6. 2. 2. 1.

Sheet 12 of 25

END-OF-PERIOD FLOW													
NO. DA	MAJN	PERIOD	RAIN	EXCS	LOSS	COMP 8	NO. DA	MAJN	PERIOD	RATN	EXCS	LOSS	COMP 8
1.01	.20	1	.00	.00	.00	0.	1.02	1.20	76	.04	.01	.03	3.
1.01	.40	2	.00	.00	.00	0.	1.22	1.40	77	.04	.01	.03	4.
1.01	1.20	3	.00	.00	.00	0.	1.02	2.00	78	.04	.01	.03	5.
1.01	1.60	4	.00	.00	.00	0.	1.02	2.20	80	.04	.01	.03	6.
1.01	1.65	5	.00	.00	.00	0.	1.02	2.40	81	.04	.01	.03	7.
1.01	1.65	6	.00	.00	.00	0.	1.02	2.60	82	.04	.01	.03	8.
1.01	1.65	7	.00	.00	.00	0.	1.02	2.80	83	.04	.01	.03	9.
1.01	1.65	8	.00	.00	.00	0.	1.02	3.00	84	.04	.01	.03	10.
1.01	1.65	9	.00	.00	.00	0.	1.02	3.20	85	.04	.01	.03	11.
1.01	1.65	10	.00	.00	.00	0.	1.02	3.40	86	.04	.01	.03	12.
1.01	1.65	11	.00	.00	.00	0.	1.02	3.60	87	.04	.01	.03	13.
1.01	1.65	12	.00	.00	.00	0.	1.02	3.80	88	.04	.01	.03	14.
1.01	1.65	13	.00	.00	.00	0.	1.02	4.00	89	.04	.01	.03	15.
1.01	1.65	14	.00	.00	.00	0.	1.02	4.20	90	.04	.01	.03	16.
1.01	1.65	15	.00	.00	.00	0.	1.02	4.40	91	.04	.01	.03	17.
1.01	1.65	16	.00	.00	.00	0.	1.02	4.60	92	.04	.01	.03	18.
1.01	1.65	17	.00	.00	.00	0.	1.02	4.80	93	.04	.01	.03	19.
1.01	1.65	18	.00	.00	.00	0.	1.02	5.00	94	.04	.01	.03	20.
1.01	1.65	19	.00	.00	.00	0.	1.02	5.20	95	.04	.01	.03	21.
1.01	1.65	20	.00	.00	.00	0.	1.02	5.40	96	.04	.01	.03	22.
1.01	1.65	21	.00	.00	.00	0.	1.02	5.60	97	.04	.01	.03	23.
1.01	1.65	22	.00	.00	.00	0.	1.02	5.80	98	.04	.01	.03	24.
1.01	1.65	23	.00	.00	.00	0.	1.02	6.00	99	.04	.01	.03	25.

END-OF-PERIOD FLOW									
NO. DA	HR. MN	PERIOD	RAIN	EXCS	LOSS	COMP Q	HR. MN	PERIOD	RAIN
1.01	1.00	1	0.00	0.00	0.00	0.00	1.02	1.20	76
1.01	1.40	2	0.00	0.00	0.00	0.00	1.02	1.40	77
1.01	1.40	3	0.00	0.00	0.00	0.00	1.02	1.40	78
1.01	1.40	4	0.00	0.00	0.00	0.00	1.02	1.40	79
1.01	1.40	5	0.00	0.00	0.00	0.00	1.02	1.40	80
1.01	1.40	6	0.00	0.00	0.00	0.00	1.02	1.40	81
1.01	1.40	7	0.00	0.00	0.00	0.00	1.02	1.40	82
1.01	1.40	8	0.00	0.00	0.00	0.00	1.02	1.40	83
1.01	1.40	9	0.00	0.00	0.00	0.00	1.02	1.40	84
1.01	1.40	10	0.00	0.00	0.00	0.00	1.02	1.40	85
1.01	1.40	11	0.00	0.00	0.00	0.00	1.02	1.40	86
1.01	1.40	12	0.00	0.00	0.00	0.00	1.02	1.40	87
1.01	1.40	13	0.00	0.00	0.00	0.00	1.02	1.40	88
1.01	1.40	14	0.00	0.00	0.00	0.00	1.02	1.40	89
1.01	1.40	15	0.00	0.00	0.00	0.00	1.02	1.40	90
1.01	1.40	16	0.00	0.00	0.00	0.00	1.02	1.40	91
1.01	1.40	17	0.00	0.00	0.00	0.00	1.02	1.40	92
1.01	1.40	18	0.00	0.00	0.00	0.00	1.02	1.40	93
1.01	1.40	19	0.00	0.00	0.00	0.00	1.02	1.40	94
1.01	1.40	20	0.00	0.00	0.00	0.00	1.02	1.40	95
1.01	1.40	21	0.00	0.00	0.00	0.00	1.02	1.40	96
1.01	1.40	22	0.00	0.00	0.00	0.00	1.02	1.40	97
1.01	1.40	23	0.00	0.00	0.00	0.00	1.02	1.40	98
1.01	1.40	24	0.00	0.00	0.00	0.00	1.02	1.40	99
1.01	1.40	25	0.00	0.00	0.00	0.00	1.02	1.40	100
1.01	1.40	26	0.00	0.00	0.00	0.00	1.02	1.40	101
1.01	1.40	27	0.00	0.00	0.00	0.00	1.02	1.40	102
1.01	1.40	28	0.00	0.00	0.00	0.00	1.02	1.40	103
1.01	1.40	29	0.00	0.00	0.00	0.00	1.02	1.40	104
1.01	1.40	30	0.00	0.00	0.00	0.00	1.02	1.40	105
1.01	1.40	31	0.00	0.00	0.00	0.00	1.02	1.40	106
1.01	1.40	32	0.00	0.00	0.00	0.00	1.02	1.40	107
1.01	1.40	33	0.00	0.00	0.00	0.00	1.02	1.40	108
1.01	1.40	34	0.00	0.00	0.00	0.00	1.02	1.40	109
1.01	1.40	35	0.00	0.00	0.00	0.00	1.02	1.40	110
1.01	1.40	36	0.00	0.00	0.00	0.00	1.02	1.40	111
1.01	1.40	37	0.00	0.00	0.00	0.00	1.02	1.40	112
1.01	1.40	38	0.00	0.00	0.00	0.00	1.02	1.40	113
1.01	1.40	39	0.00	0.00	0.00	0.00	1.02	1.40	114
1.01	1.40	40	0.00	0.00	0.00	0.00	1.02	1.40	115
1.01	1.40	41	0.00	0.00	0.00	0.00	1.02	1.40	116
1.01	1.40	42	0.00	0.00	0.00	0.00	1.02	1.40	117
1.01	1.40	43	0.00	0.00	0.00	0.00	1.02	1.40	118
1.01	1.40	44	0.00	0.00	0.00	0.00	1.02	1.40	119
1.01	1.40	45	0.00	0.00	0.00	0.00	1.02	1.40	120
1.01	1.40	46	0.00	0.00	0.00	0.00	1.02	1.40	121
1.01	1.40	47	0.00	0.00	0.00	0.00	1.02	1.40	122
1.01	1.40	48	0.00	0.00	0.00	0.00	1.02	1.40	123
1.01	1.40	49	0.00	0.00	0.00	0.00	1.02	1.40	124
1.01	1.40	50	0.00	0.00	0.00	0.00	1.02	1.40	125
1.01	1.40	51	0.00	0.00	0.00	0.00	1.02	1.40	126
1.01	1.40	52	0.00	0.00	0.00	0.00	1.02	1.40	127
1.01	1.40	53	0.00	0.00	0.00	0.00	1.02	1.40	128
1.01	1.40	54	0.00	0.00	0.00	0.00	1.02	1.40	129
1.01	1.40	55	0.00	0.00	0.00	0.00	1.02	1.40	130
1.01	1.40	56	0.00	0.00	0.00	0.00	1.02	1.40	131

Sheet 13 of 35

1.01	1.40	57	0.00	0.00	0.00	0.00	1.02	1.40	132
1.01	1.40	58	0.00	0.00	0.00	0.00	1.02	1.40	133
1.01	1.40	59	0.00	0.00	0.00	0.00	1.02	1.40	134
1.01	1.40	60	0.00	0.00	0.00	0.00	1.02	1.40	135
1.01	1.40	61	0.00	0.00	0.00	0.00	1.02	1.40	136
1.01	1.40	62	0.00	0.00	0.00	0.00	1.02	1.40	137
1.01	1.40	63	0.00	0.00	0.00	0.00	1.02	1.40	138
1.01	1.40	64	0.00	0.00	0.00	0.00	1.02	1.40	139
1.01	1.40	65	0.00	0.00	0.00	0.00	1.02	1.40	140











STATION 2, FLAN 7, RATIO 2

END-OF-PERIOD HYDROGRAPH ORDINATES

[illegible][illegible][illegible]

PEAS OFFICE IS 51 FORTALINE ROAD 469. AT TIME 42.67 HOURS

PEAK	6-HOUR	24-HOUR	72-HOUR	TOTAL VOLUME
1	1.0	1.0	1.0	3.0
2	1.0	1.0	1.0	3.0
3	1.0	1.0	1.0	3.0
4	1.0	1.0	1.0	3.0
5	1.0	1.0	1.0	3.0
6	1.0	1.0	1.0	3.0
7	1.0	1.0	1.0	3.0
8	1.0	1.0	1.0	3.0
9	1.0	1.0	1.0	3.0
10	1.0	1.0	1.0	3.0
11	1.0	1.0	1.0	3.0
12	1.0	1.0	1.0	3.0
13	1.0	1.0	1.0	3.0
14	1.0	1.0	1.0	3.0
15	1.0	1.0	1.0	3.0
16	1.0	1.0	1.0	3.0
17	1.0	1.0	1.0	3.0
18	1.0	1.0	1.0	3.0
19	1.0	1.0	1.0	3.0
20	1.0	1.0	1.0	3.0
21	1.0	1.0	1.0	3.0
22	1.0	1.0	1.0	3.0
23	1.0	1.0	1.0	3.0
24	1.0	1.0	1.0	3.0
25	1.0	1.0	1.0	3.0
26	1.0	1.0	1.0	3.0
27	1.0	1.0	1.0	3.0
28	1.0	1.0	1.0	3.0
29	1.0	1.0	1.0	3.0
30	1.0	1.0	1.0	3.0
31	1.0	1.0	1.0	3.0
32	1.0	1.0	1.0	3.0
33	1.0	1.0	1.0	3.0
34	1.0	1.0	1.0	3.0
35	1.0	1.0	1.0	3.0
36	1.0	1.0	1.0	3.0
37	1.0	1.0	1.0	3.0
38	1.0	1.0	1.0	3.0
39	1.0	1.0	1.0	3.0
40	1.0	1.0	1.0	3.0
41	1.0	1.0	1.0	3.0
42	1.0	1.0	1.0	3.0
43	1.0	1.0	1.0	3.0
44	1.0	1.0	1.0	3.0
45	1.0	1.0	1.0	3.0
46	1.0	1.0	1.0	3.0
47	1.0	1.0	1.0	3.0
48	1.0	1.0	1.0	3.0
49	1.0	1.0	1.0	3.0
50	1.0	1.0	1.0	3.0
51	1.0	1.0	1.0	3.0
52	1.0	1.0	1.0	3.0
53	1.0	1.0	1.0	3.0
54	1.0	1.0	1.0	3.0
55	1.0	1.0	1.0	3.0
56	1.0	1.0	1.0	3.0
57	1.0	1.0	1.0	3.0
58	1.0	1.0	1.0	3.0
59	1.0	1.0	1.0	3.0
60	1.0	1.0	1.0	3.0
61	1.0	1.0	1.0	3.0
62	1.0	1.0	1.0	3.0
63	1.0	1.0	1.0	3.0
64	1.0	1.0	1.0	3.0
65	1.0	1.0	1.0	3.0
66	1.0	1.0	1.0	3.0
67	1.0	1.0	1.0	3.0
68	1.0	1.0	1.0	3.0
69	1.0	1.0	1.0	3.0
70	1.0	1.0	1.0	3.0
71	1.0	1.0	1.0	3.0
72	1.0	1.0	1.0	3.0
73	1.0	1.0	1.0	3.0
74	1.0	1.0	1.0	3.0
75	1.0	1.0	1.0	3.0
76	1.0	1.0	1.0	3.0
77	1.0	1.0	1.0	3.0
78	1.0	1.0	1.0	3.0
79	1.0	1.0	1.0	3.0
80	1.0	1.0	1.0	3.0

Sheet 15 of 25

CFS	60.	342.	104.	50.	7544.
CFS	20.	10.	3.	1.	214.
INCHES		12.25	14.82	15.00	15.00
MM		311.21	377.96	380.87	380.87
AC-FI		170.	206.	206.	208.
THOUS CU FT		209.	254.	256.	256.















STAGE	6-HOUR	24-HOUR	72-HOUR	TOTAL VOLUME
474.0	476.0	476.0	476.0	476.0
475.0	476.0	476.0	476.0	476.0
476.0	476.0	476.0	476.0	476.0
477.0	476.0	476.0	476.0	476.0
478.0	476.0	476.0	476.0	476.0
479.0	476.0	476.0	476.0	476.0
480.0	476.0	476.0	476.0	476.0
481.0	476.0	476.0	476.0	476.0
482.0	476.0	476.0	476.0	476.0
483.0	476.0	476.0	476.0	476.0
484.0	476.0	476.0	476.0	476.0
485.0	476.0	476.0	476.0	476.0
486.0	476.0	476.0	476.0	476.0
487.0	476.0	476.0	476.0	476.0
488.0	476.0	476.0	476.0	476.0
489.0	476.0	476.0	476.0	476.0
490.0	476.0	476.0	476.0	476.0
491.0	476.0	476.0	476.0	476.0
492.0	476.0	476.0	476.0	476.0
493.0	476.0	476.0	476.0	476.0
494.0	476.0	476.0	476.0	476.0
495.0	476.0	476.0	476.0	476.0
496.0	476.0	476.0	476.0	476.0
497.0	476.0	476.0	476.0	476.0
498.0	476.0	476.0	476.0	476.0
499.0	476.0	476.0	476.0	476.0
500.0	476.0	476.0	476.0	476.0

MAXIMUM STORAGE = 0.

TABLE 15 472.5

STATION	PLAN 1, REID 4	OUTFLOW
0	0	0
1	0	0
2	0	0
3	0	0
4	0	0
5	0	0
6	0	0
7	0	0
8	0	0
9	0	0
10	0	0
11	0	0
12	0	0
13	0	0
14	0	0
15	0	0
16	0	0
17	0	0
18	0	0
19	0	0
20	0	0
21	0	0
22	0	0
23	0	0
24	0	0
25	0	0
26	0	0
27	0	0
28	0	0
29	0	0
30	0	0
31	0	0
32	0	0
33	0	0
34	0	0
35	0	0
36	0	0
37	0	0
38	0	0
39	0	0
40	0	0
41	0	0
42	0	0
43	0	0
44	0	0
45	0	0
46	0	0
47	0	0
48	0	0
49	0	0
50	0	0

500.0 REID 4



[illegible]









DATE	TIME	6-HOUR	24-HOUR	72-HOUR	TOTAL VOLUME
1-11-68	141	543	106	96	1447
1-12-68	103	11	6	43	160
1-13-68	104	17	21	21	59
1-14-68	104	43	53	54	150
1-15-68	104	110	303	397	810
1-16-68	104	303	485	490	1278

DATE	TIME	6-HOUR	24-HOUR	72-HOUR	TOTAL VOLUME
1-17-68	104	106	106	106	318
1-18-68	104	106	106	106	318
1-19-68	104	106	106	106	318
1-20-68	104	106	106	106	318
1-21-68	104	106	106	106	318
1-22-68	104	106	106	106	318
1-23-68	104	106	106	106	318
1-24-68	104	106	106	106	318
1-25-68	104	106	106	106	318
1-26-68	104	106	106	106	318
1-27-68	104	106	106	106	318
1-28-68	104	106	106	106	318
1-29-68	104	106	106	106	318
1-30-68	104	106	106	106	318
1-31-68	104	106	106	106	318

DATE	TIME	6-HOUR	24-HOUR	72-HOUR	TOTAL VOLUME
2-1-68	104	106	106	106	318
2-2-68	104	106	106	106	318
2-3-68	104	106	106	106	318
2-4-68	104	106	106	106	318
2-5-68	104	106	106	106	318
2-6-68	104	106	106	106	318
2-7-68	104	106	106	106	318
2-8-68	104	106	106	106	318
2-9-68	104	106	106	106	318
2-10-68	104	106	106	106	318
2-11-68	104	106	106	106	318
2-12-68	104	106	106	106	318
2-13-68	104	106	106	106	318
2-14-68	104	106	106	106	318
2-15-68	104	106	106	106	318
2-16-68	104	106	106	106	318
2-17-68	104	106	106	106	318
2-18-68	104	106	106	106	318
2-19-68	104	106	106	106	318
2-20-68	104	106	106	106	318
2-21-68	104	106	106	106	318
2-22-68	104	106	106	106	318
2-23-68	104	106	106	106	318
2-24-68	104	106	106	106	318
2-25-68	104	106	106	106	318
2-26-68	104	106	106	106	318
2-27-68	104	106	106	106	318
2-28-68	104	106	106	106	318
2-29-68	104	106	106	106	318
2-30-68	104	106	106	106	318
2-31-68	104	106	106	106	318

7-10-68

DATE	TIME	6-HOUR	24-HOUR	72-HOUR	TOTAL VOLUME
3-1-68	104	106	106	106	318
3-2-68	104	106	106	106	318
3-3-68	104	106	106	106	318
3-4-68	104	106	106	106	318
3-5-68	104	106	106	106	318
3-6-68	104	106	106	106	318
3-7-68	104	106	106	106	318
3-8-68	104	106	106	106	318
3-9-68	104	106	106	106	318
3-10-68	104	106	106	106	318
3-11-68	104	106	106	106	318
3-12-68	104	106	106	106	318
3-13-68	104	106	106	106	318
3-14-68	104	106	106	106	318
3-15-68	104	106	106	106	318
3-16-68	104	106	106	106	318
3-17-68	104	106	106	106	318
3-18-68	104	106	106	106	318
3-19-68	104	106	106	106	318
3-20-68	104	106	106	106	318
3-21-68	104	106	106	106	318
3-22-68	104	106	106	106	318
3-23-68	104	106	106	106	318
3-24-68	104	106	106	106	318
3-25-68	104	106	106	106	318
3-26-68	104	106	106	106	318
3-27-68	104	106	106	106	318
3-28-68	104	106	106	106	318
3-29-68	104	106	106	106	318
3-30-68	104	106	106	106	318
3-31-68	104	106	106	106	318





[illegible]











ELEVATIONS	470.	474.	476.	480.
5-EL	474.0	474.0	476.0	480.0
COGN	0.0	0.0	0.0	0.0
EXPW	0.0	0.0	0.0	0.0
FLEV	0.0	0.0	0.0	0.0
CCOL	0.0	0.0	0.0	0.0
CASEA	0.0	0.0	0.0	0.0
EXPL	0.0	0.0	0.0	0.0
TOPEL	476.0	476.0	476.0	476.0
COGN	3.1	3.1	3.1	3.1
EXPW	1.5	1.5	1.5	1.5
DEWID	365.	365.	365.	365.
STATION	10.	FLAN 1.	RATIO 1	
100-OF-PERIOD HYDROGRAPH ORDINATES				
OUTFLOW				
1	0.	0.	0.	0.
2	0.	0.	0.	0.
3	0.	0.	0.	0.
4	0.	0.	0.	0.
5	1.	1.	1.	1.
6	3.	3.	3.	3.
7	4.	4.	4.	4.
8	4.	4.	4.	4.
9	4.	4.	4.	4.
10	4.	4.	4.	4.
11	4.	4.	4.	4.
12	4.	4.	4.	4.
13	4.	4.	4.	4.
14	4.	4.	4.	4.
15	4.	4.	4.	4.
16	4.	4.	4.	4.
17	4.	4.	4.	4.
18	4.	4.	4.	4.
19	4.	4.	4.	4.
20	4.	4.	4.	4.
21	4.	4.	4.	4.
22	4.	4.	4.	4.
23	4.	4.	4.	4.
24	4.	4.	4.	4.
25	4.	4.	4.	4.
26	4.	4.	4.	4.
27	4.	4.	4.	4.
28	4.	4.	4.	4.
29	4.	4.	4.	4.
30	4.	4.	4.	4.
31	4.	4.	4.	4.
32	4.	4.	4.	4.
33	4.	4.	4.	4.
34	4.	4.	4.	4.
35	4.	4.	4.	4.
36	4.	4.	4.	4.
37	4.	4.	4.	4.
38	4.	4.	4.	4.
39	4.	4.	4.	4.
40	4.	4.	4.	4.
41	4.	4.	4.	4.
42	4.	4.	4.	4.
43	4.	4.	4.	4.
44	4.	4.	4.	4.
45	4.	4.	4.	4.
46	4.	4.	4.	4.
47	4.	4.	4.	4.
48	4.	4.	4.	4.
49	4.	4.	4.	4.
50	4.	4.	4.	4.
51	4.	4.	4.	4.
52	4.	4.	4.	4.
53	4.	4.	4.	4.
54	4.	4.	4.	4.
55	4.	4.	4.	4.
56	4.	4.	4.	4.
57	4.	4.	4.	4.
58	4.	4.	4.	4.
59	4.	4.	4.	4.
60	4.	4.	4.	4.
61	4.	4.	4.	4.
62	4.	4.	4.	4.
63	4.	4.	4.	4.
64	4.	4.	4.	4.
65	4.	4.	4.	4.
66	4.	4.	4.	4.
67	4.	4.	4.	4.
68	4.	4.	4.	4.
69	4.	4.	4.	4.
70	4.	4.	4.	4.
71	4.	4.	4.	4.
72	4.	4.	4.	4.
73	4.	4.	4.	4.
74	4.	4.	4.	4.
75	4.	4.	4.	4.
76	4.	4.	4.	4.
77	4.	4.	4.	4.
78	4.	4.	4.	4.
79				

AD-A107 410

TIPPETIS-ABBETT-MCCARTHY-STRATTON NEW YORK

F/6 13/13

NATIONAL DAM SAFETY PROGRAM. LONG POND DAM NUMBER 3 (INVENTORY --ETC(U)

SEP 81 E O'BRIEN

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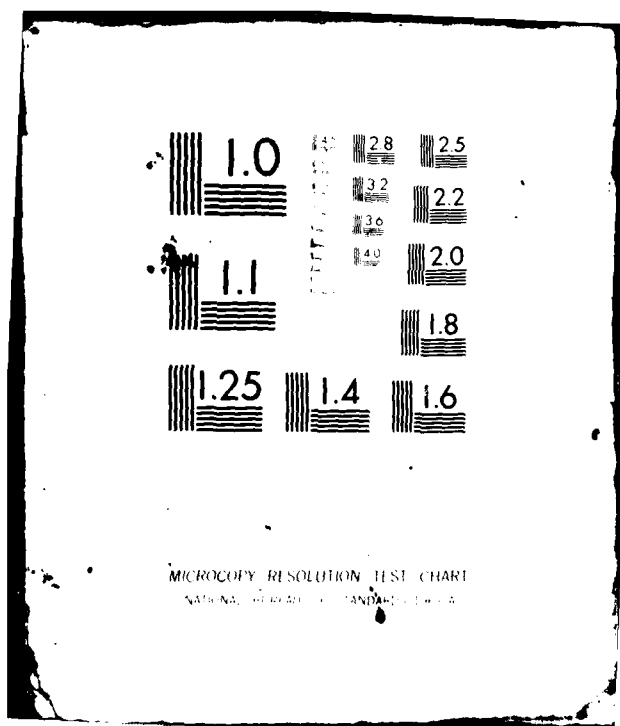
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PEAK FLOW AND STORAGE (END OF PERIOD) SUMMARY FOR MULTIPLE PLAN-RATIO ECONOMIC COMPUTATIONS  
 FLOWS IN CUBIC FEET PER SECOND (CUBIC METERS PER SECOND)  
 AREA IN SQUARE MILES (SQUARE KILOMETERS)

OPERATION	STATION	AREA	PLAN RATIO	RATIOS APPLIED TO FLOWS			
				1 RATIO	2 RATIO	3 RATIO	4 RATIO
				1.20	.75	.50	.25
HYDROGRAPH AT	1	.26 (.67)	1	923. (26.12)	692. (19.59)	467. (13.05)	231. (6.53)
ROUTED TO	2	.26 (.67)	1	923. (26.13)	690. (19.53)	456. (12.91)	143. (4.06)
ROUTED TO	3	.26 (.67)	1	921. (26.07)	691. (19.55)	457. (12.77)	144. (4.09)
HYDROGRAPH AT	2	.29 (.73)	1	361. (10.20)	273. (7.72)	182. (5.15)	91. (2.57)
2 COMBINED	2	.35 (.91)	1	1224. (34.66)	916. (25.99)	631. (17.62)	197. (5.50)
HYDROGRAPH AT	3	.41 (.98)	1	433. (12.25)	324. (9.19)	215. (6.17)	105. (3.00)
2 COMBINED	3	.49 (1.19)	1	1647. (46.54)	1224. (34.66)	806. (22.83)	242. (6.85)
ROUTED TO	10	.66 (1.59)	1	1630. (46.15)	1214. (34.38)	761. (21.55)	159. (4.50)

Sheet 44 of 35

NORTH LAKE DAM  
 SUMMARY OF DAM SAFETY ANALYSIS

PLAN 1	ELEVATION	INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
	STORAGE	572.00	572.00	573.75
		210.	210.	249.

# MOG-VALE DAM

## SUMMARY OF DAM SAFETY ANALYSIS

PLAN 1	ELEVATION STORAGE OUTFLOW	INITIAL VALUE 572.00 210. 0.	SPILLWAY CREST 572.00 210. 0.	TOP OF DAM 573.75 249. 70.
--------	---------------------------------	---------------------------------------	--	-------------------------------------

RATIO OF O/F	MAXIMUM RESENCIOR W.S. ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1.00	574.37	.62	263.	923.	8.67	40.67	0.00
.75	574.25	.50	260.	690.	6.33	40.67	0.00
.50	574.11	.36	257.	456.	4.67	40.67	0.00
.25	573.86	.11	251.	143.	2.33	41.67	0.00

## PLAN 1 STATION 3

RATIO	MAXIMUM FLOW, CFS	MAXIMUM STAGE, FT	TIME HOURS
1.00	923.	478.7	40.67
.75	691.	478.3	40.67
.50	450.	477.8	40.67
.25	143.	477.0	41.67

Sheet 45 of 85

# LONG POINT DAM w/o Auxiliary Spillway

## SUMMARY OF DAM SAFETY ANALYSIS

PLAN 1	ELEVATION STORAGE OUTFLOW	INITIAL VALUE 470.00 115. 0.	SPILLWAY CREST 470.00 115. 0.	TOP OF DAM 476.00 174. 227.
--------	---------------------------------	---------------------------------------	--	--------------------------------------

LONG POND DAM w/o Auxiliary Spillway

SUMMARY OF DAM SAFETY ANALYSIS

PLAN 1									
ELEVATION		INITIAL VALUE		SPILLWAY CREST		TOP OF DAM			
STORAGE		470.00		470.00		476.00			
OUTFLOW		115.		115.		174.			
		0.		0.		222.			
RATIO OF PMF	MAXIMUM RESERVOIR WATERLEVEL	MAXIMUM DEPTH OVER DAM		MAXIMUM STORAGE AC-FT		MAXIMUM OUTFLOW CFS		DURATION OVER TOP HOURS	
		TIME OF FAILURE HOURS		TIME OF MAX OUTFLOW HOURS		TIME OF FAILURE HOURS			
1.00	477.14	1.14		188.		1630.		5.00	
.75	476.93	.90		185.		1214.		4.00	
.50	476.60	.60		181.		761.		3.00	
.25	474.30	0.00		155.		159.		0.00	

Sheet 4 of 20

FLOOD HYDROGRAPH PREPARE (DEC-7)  
 DAM SAFETY VERSION JULY 1976  
 LAST MODIFICATION 01 APR 80

WITH HIGHWAY EMPLOYMENT  
 AS ESTABLISHED IN 1976

# CASE II

LONG POND DAM NO 3  
 COURSE 1 SAFETY INSPECTION  
 DEC 1 06 PMF ANALYSIS

1 11 1 0.75 0.5 0.25

2 12 1 0.75 0.5 0.25

3 13 1 0.75 0.5 0.25

4 14 1 0.75 0.5 0.25

5 15 1 0.75 0.5 0.25

6 16 1 0.75 0.5 0.25

7 17 1 0.75 0.5 0.25

8 18 1 0.75 0.5 0.25

9 19 1 0.75 0.5 0.25

10 20 1 0.75 0.5 0.25

11 21 1 0.75 0.5 0.25

12 22 1 0.75 0.5 0.25

13 23 1 0.75 0.5 0.25

14 24 1 0.75 0.5 0.25

15 25 1 0.75 0.5 0.25

16 26 1 0.75 0.5 0.25

17 27 1 0.75 0.5 0.25

18 28 1 0.75 0.5 0.25

19 29 1 0.75 0.5 0.25

20 30 1 0.75 0.5 0.25

21 31 1 0.75 0.5 0.25

22 32 1 0.75 0.5 0.25

23 33 1 0.75 0.5 0.25

24 34 1 0.75 0.5 0.25

25 35 1 0.75 0.5 0.25

26 36 1 0.75 0.5 0.25

27 37 1 0.75 0.5 0.25

28 38 1 0.75 0.5 0.25

29 39 1 0.75 0.5 0.25

30 40 1 0.75 0.5 0.25

31 41 1 0.75 0.5 0.25

32 42 1 0.75 0.5 0.25

33 43 1 0.75 0.5 0.25

34 44 1 0.75 0.5 0.25

35 45 1 0.75 0.5 0.25

36 46 1 0.75 0.5 0.25

37 47 1 0.75 0.5 0.25

38 48 1 0.75 0.5 0.25

39 49 1 0.75 0.5 0.25

40 50 1 0.75 0.5 0.25

41 51 1 0.75 0.5 0.25

42 52 1 0.75 0.5 0.25

43 53 1 0.75 0.5 0.25

44 54 1 0.75 0.5 0.25

45 55 1 0.75 0.5 0.25

46 56 1 0.75 0.5 0.25

47 57 1 0.75 0.5 0.25

48 58 1 0.75 0.5 0.25

49 59 1 0.75 0.5 0.25

50 60 1 0.75 0.5 0.25

1 11 1 0.75 0.5 0.25

1 11 1 0.75 0.5 0.25

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1 11 1 0.75 0.5 0.25

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1 11 1 0.75 0.5 0.25

1 11 1 0.75 0.5 0.25

1 11 1 0.75 0.5 0.25

1 11 1 0.75 0.5 0.25

ST	K1	1U ROUTE THROUGH LONG POND (INCLUDING FLOW OVER DEPRESSION)	1	1	1	1
51	V					
52	V1	1				115
53	V4	470	471.2	472	475	475.5
54	V5	115	115	115	115	115
55	V6	115	115	115	115	115
56	V7	115	115	115	115	115
57	V8	115	115	115	115	115
58	V9	115	115	115	115	115
59	V10	115	115	115	115	115
60	V11	115	115	115	115	115
61	V12	115	115	115	115	115
62	V13	115	115	115	115	115
63	V14	115	115	115	115	115
64	V15	115	115	115	115	115
65	V16	115	115	115	115	115
66	V17	115	115	115	115	115
67	V18	115	115	115	115	115
68	V19	115	115	115	115	115
69	V20	115	115	115	115	115
70	V21	115	115	115	115	115
71	V22	115	115	115	115	115
72	V23	115	115	115	115	115
73	V24	115	115	115	115	115
74	V25	115	115	115	115	115
75	V26	115	115	115	115	115
76	V27	115	115	115	115	115
77	V28	115	115	115	115	115
78	V29	115	115	115	115	115
79	V30	115	115	115	115	115
80	V31	115	115	115	115	115
81	V32	115	115	115	115	115
82	V33	115	115	115	115	115
83	V34	115	115	115	115	115
84	V35	115	115	115	115	115
85	V36	115	115	115	115	115
86	V37	115	115	115	115	115
87	V38	115	115	115	115	115
88	V39	115	115	115	115	115
89	V40	115	115	115	115	115
90	V41	115	115	115	115	115
91	V42	115	115	115	115	115
92	V43	115	115	115	115	115
93	V44	115	115	115	115	115
94	V45	115	115	115	115	115
95	V46	115	115	115	115	115
96	V47	115	115	115	115	115
97	V48	115	115	115	115	115
98	V49	115	115	115	115	115
99	V50	115	115	115	115	115
100	V51	115	115	115	115	115

ANALYSIS OF STRAIN RATE EFFECTS

PREVIEW OF SEQUENCE OF STREAM NETWORK CALCULATIONS

RUNOFF HYDROGRAPH AT	1
ROUTE HYDROGRAPH TO	2
RUNOFF HYDROGRAPH TO	3
RUNOFF HYDROGRAPH AT	2
CONJUNCTIVE 2 HYDROGRAPHS AT	2
ROUTE HYDROGRAPH AT	3
CONJUNCTIVE 3 HYDROGRAPHS AT	3
ROUTE HYDROGRAPH TO	10
END OF METHOD	

\*\*\*\*\*  
 FLOOD HYDROGRAPH PACKAGE (HEC-1)  
 DAM SAFETY VERSION JULY 1978  
 LAST MODIFICATION 01 APR 80  
 \*\*\*\*\*

RUN DATE 01/06/20  
 TIME 12.49.20

LONG POND DAM NO 3  
 PHASE 1 SAFETY INSPECTION  
 HEC 1 DC PMF ANALYSIS

JOB SPECIFICATION									
NQ	NHR	NMIN	IDAY	IHR	IMIN	PETRC	IPLT	JPRT	INSTAN
150	0	20	0	0	0	0	0	0	0
JOPER NWT LROPT TRACT									
5 0 0 0									

MULTI-PLAN ANALYSES TO BE PERFORMED

RTIOS= 1.00 .75 .50 .25  
 HPLAN= 1 RTIO= 4 LRTIO= 1

SUB-AREA PUNOFF COMPUTATION

1 NORTH LAKE BASIN RUN-OFF

ISTAQ	ICOMP	IECON	ITAPE	JPLT	JPRT	INAME	ISTAGE	TAUTO
1	0	0	0	0	0	1	0	0

HYDROGRAPH DATA

IMYR	IUNG	TAREA	SVAP	TPSDA	TPSPC	PATIO	ISNOW	ISAME	LOCAL
1	1	.26	0.00	.46	0.00	0.000	0	1	0

PRECIP DATA

SPFE	PMS	R6	R12	R24	R48	R72	R96
0.00	22.00	112.00	123.00	133.00	141.00	0.00	0.00

TPSPC COMPUTED BY THE PROGRAM IS .60

LOSS DATA

LROPT	STRKR	DLTKR	PTOL	FRIN	STRES	RTOK	SPTL	CNSTL	ALSMX	RTIMP
0	0.00	0.00	1.00	0.00	0.00	1.00	1.00	1.0	0.00	.13

UNIT HYDROGRAPH DATA

IP= .95 CP= .63 NTA= 0

RECESSION DATA

STRIO= -1.00 ORCSH= .10 RTIO= 1.50  
 APPROPRIATE CLARK COEFFICIENTS FROM GIVEN SWYDER CP AND TP ARE 3.63 AND R= 2.36 INTERVALS

UNIT HYDROGRAPH 15 END-OF-PERIOD COORDINATES, LAG= .97 HOURS, CP= .62 VOL= 1.00  
 18. 62. 101. 76. 50. 32. 21. 14. 9.  
 6. 4. 2. 1.

Sheet 50 of 85

\*\*\*\*\*  
 END-OF-PERIOD PRINT  
 \*\*\*\*\*

# RECESSION DATA

APPROXIMATE CORRECTION FACTORS FROM GIVEN SNYDER CP AND TP ARE TC= 3.63 AND R= 2.36 INTERVALS

UNIT HYDROGRAPH IS END-OF-PERIOD ORDINATES, LAG= .97 HOURS, CFS= .62 VOL= 1.00

NO. DA	HR. PN	PERIOD	RAIN	EXCS	LOSS	COMP Q	MO. DA	HR. MW	PERIOD	RAIN	EXCS	LOSS	COMP Q
1.01	1.20	1	.00	.00	.00	0.	1.02	1.20	76	.04	.01	.03	3.
1.01	1.40	2	.00	.00	.00	0.	1.02	1.40	77	.04	.01	.03	4.
1.01	1.60	3	.00	.00	.00	0.	1.02	2.00	78	.04	.01	.03	5.
1.01	1.80	4	.00	.00	.00	0.	1.02	2.20	79	.04	.01	.03	5.
1.01	2.00	5	.00	.00	.00	0.	1.02	2.40	80	.04	.01	.03	5.
1.01	2.20	6	.00	.00	.00	0.	1.02	2.60	81	.04	.01	.03	5.
1.01	2.40	7	.00	.00	.00	0.	1.02	2.80	82	.04	.01	.03	5.
1.01	2.60	8	.00	.00	.00	0.	1.02	3.00	83	.04	.01	.03	5.
1.01	2.80	9	.00	.00	.00	0.	1.02	3.20	84	.04	.01	.03	5.
1.01	3.00	10	.00	.00	.00	0.	1.02	3.40	85	.04	.01	.03	5.
1.01	3.20	11	.00	.00	.00	0.	1.02	3.60	86	.04	.01	.03	5.
1.01	3.40	12	.00	.00	.00	0.	1.02	3.80	87	.04	.01	.03	5.
1.01	3.60	13	.00	.00	.00	0.	1.02	4.00	88	.04	.01	.03	5.
1.01	3.80	14	.00	.00	.00	0.	1.02	4.20	89	.04	.01	.03	5.
1.01	4.00	15	.00	.00	.00	0.	1.02	4.40	90	.04	.01	.03	5.
1.01	4.20	16	.00	.00	.00	0.	1.02	4.60	91	.04	.01	.03	5.
1.01	4.40	17	.00	.00	.00	0.	1.02	4.80	92	.04	.01	.03	5.
1.01	4.60	18	.00	.00	.00	0.	1.02	5.00	93	.04	.01	.03	5.
1.01	4.80	19	.00	.00	.00	0.	1.02	5.20	94	.04	.01	.03	5.
1.01	5.00	20	.00	.00	.00	0.	1.02	5.40	95	.04	.01	.03	5.
1.01	5.20	21	.00	.00	.00	0.	1.02	5.60	96	.04	.01	.03	5.
1.01	5.40	22	.00	.00	.00	0.	1.02	5.80	97	.04	.01	.03	5.
1.01	5.60	23	.00	.00	.00	0.	1.02	6.00	98	.04	.01	.03	5.
1.01	5.80	24	.00	.00	.00	0.	1.02	6.20	99	.04	.01	.03	5.
1.01	6.00	25	.00	.00	.00	0.	1.02	6.40	100	.04	.01	.03	5.
1.01	6.20	26	.00	.00	.00	0.	1.02	6.60	101	.04	.01	.03	5.
1.01	6.40	27	.00	.00	.00	0.	1.02	6.80	102	.04	.01	.03	5.
1.01	6.60	28	.00	.00	.00	0.	1.02	7.00	103	.04	.01	.03	5.
1.01	6.80	29	.00	.00	.00	0.	1.02	7.20	104	.04	.01	.03	5.
1.01	7.00	30	.00	.00	.00	0.	1.02	7.40	105	.04	.01	.03	5.
1.01	7.20	31	.00	.00	.00	0.	1.02	7.60	106	.04	.01	.03	5.
1.01	7.40	32	.00	.00	.00	0.	1.02	7.80	107	.04	.01	.03	5.
1.01	7.60	33	.00	.00	.00	0.	1.02	8.00	108	.04	.01	.03	5.
1.01	7.80	34	.00	.00	.00	0.	1.02	8.20	109	.04	.01	.03	5.
1.01	8.00	35	.00	.00	.00	0.	1.02	8.40	110	.04	.01	.03	5.
1.01	8.20	36	.00	.00	.00	0.	1.02	8.60	111	.04	.01	.03	5.
1.01	8.40	37	.00	.00	.00	0.	1.02	8.80	112	.04	.01	.03	5.
1.01	8.60	38	.00	.00	.00	0.	1.02	9.00	113	.04	.01	.03	5.
1.01	8.80	39	.00	.00	.00	0.	1.02	9.20	114	.04	.01	.03	5.
1.01	9.00	40	.00	.00	.00	0.	1.02	9.40	115	.04	.01	.03	5.
1.01	9.20	41	.00	.00	.00	0.	1.02	9.60	116	.04	.01	.03	5.
1.01	9.40	42	.00	.00	.00	0.	1.02	9.80	117	.04	.01	.03	5.
1.01	9.60	43	.00	.00	.00	0.	1.02	10.00	118	.04	.01	.03	5.
1.01	9.80	44	.00	.00	.00	0.	1.02	10.20	119	.04	.01	.03	5.
1.01	10.00	45	.00	.00	.00	0.	1.02	10.40	120	.04	.01	.03	5.
1.01	10.20	46	.00	.00	.00	0.	1.02	10.60	121	.04	.01	.03	5.
1.01	10.40	47	.00	.00	.00	0.	1.02	10.80	122	.04	.01	.03	5.
1.01	10.60	48	.00	.00	.00	0.	1.02	11.00	123	.04	.01	.03	5.
1.01	10.80	49	.00	.00	.00	0.	1.02	11.20	124	.04	.01	.03	5.
1.01	11.00	50	.00	.00	.00	0.	1.02	11.40	125	.04	.01	.03	5.
1.01	11.20	51	.00	.00	.00	0.	1.02	11.60	126	.04	.01	.03	5.
1.01	11.40	52	.00	.00	.00	0.	1.02	11.80	127	.04	.01	.03	5.
1.01	11.60	53	.00	.00	.00	0.	1.02	12.00	128	.04	.01	.03	5.
1.01	11.80	54	.00	.00	.00	0.	1.02	12.20	129	.04	.01	.03	5.
1.01	12.00	55	.00	.00	.00	0.	1.02	12.40	130	.04	.01	.03	5.
1.01	12.20	56	.00	.00	.00	0.	1.02	12.60	131	.04	.01	.03	5.





INCHES 17.90 22.78 23.12 23.12  
 THOUS CU M 178 306 329 305

HYDROGRAPH AT STA 1 FOR PLAN 1, RTIO 2

PEAK	6-HOUR	24-HOUR	72-HOUR	TOTAL VOLUME
462	375	119	58	8722
20	11	3	2	247
	13.43	17.09	17.34	17.34
	341.09	436.02	440.36	440.36
	186	237	240	240
	230	292	296	296

CFS

CFS

INCHES

AC-FT

THOUS CU M

HYDROGRAPH AT STA 1 FOR PLAN 1, RTIO 3

PEAK	6-HOUR	24-HOUR	72-HOUR	TOTAL VOLUME
461	257	80	39	5815
13	7	2	1	165
	8.95	11.39	11.56	11.56
	227.39	289.34	293.58	293.58
	124	160	160	160
	153	195	198	198

CFS

CFS

INCHES

AC-FT

THOUS CU M

HYDROGRAPH AT STA 1 FOR PLAN 1, RTIO 4

PEAK	6-HOUR	24-HOUR	72-HOUR	TOTAL VOLUME
461	257	80	39	5815
13	7	2	1	165
	8.95	11.39	11.56	11.56
	227.39	289.34	293.58	293.58
	124	160	160	160
	153	195	198	198

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STORAGE	0.00	.26	.74	1.41	2.26	3.25	4.36	5.60	6.96	8.46
	10.03	11.34	13.75	15.87	18.19	20.72	23.45	26.39	29.53	32.88

STORAGE	0.00	.26	.74	1.41	2.26	3.25	4.36	5.60	6.96	8.46
	10.03	11.34	13.75	15.87	18.19	20.72	23.45	26.39	29.53	32.88

OUTFLOW	0.00	191.07	776.04	1971.51	3892.90	6684.99	10228.09	14560.86	19722.81	25753.83
	32693.51	40382.41	49269.75	59032.24	70134.11	82581.95	96446.30	111798.25	128708.21	147245.45

STAGE	476.00	477.26	478.53	479.79	481.05	482.32	483.58	484.84	486.11	487.37
	488.63	489.89	491.16	492.42	493.68	494.95	496.21	497.47	498.74	500.00

FLOW	0.00	191.07	776.04	1874.51	3852.90	6684.99	10228.09	14560.86	19722.81	25793.83
FLOW	32643.81	40382.41	49200.75	59036.24	70134.11	82591.95	96466.30	111799.25	128708.21	147245.45

STATION 3, PL 1, R110 1

0.	0.	0.	0.	0.
0.	0.	0.	0.	0.
0.	0.	0.	0.	0.
0.	0.	0.	0.	0.
0.	0.	0.	0.	1.
0.	0.	0.	0.	2.
0.	0.	0.	0.	3.
0.	0.	0.	0.	2.
0.	0.	0.	0.	3.
0.	0.	0.	0.	4.
9.	10.	1.	1.	12.
25.	40.	25.	59.	119.
81.	921.	151.	774.	624.
35.	123.	103.	95.	85.
70.	70.	60.	65.	67.

5108

[illegible]

## STAGE

1970	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476.0	476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PRECIP. DATA

TRSPC COMPUTED BY THE PROGRAM IS .803

LOSS DATA

LROPT STRK DUTR RTIOU FRAIN STRKS RTIOU STRTL CNSTL ALSMT RTIMP

UNIT HYDROGRAPH DATA

APPROXIMATE CLARK COEFFICIENTS FROM GIVEN SNYDER CP AND TP ARE TC= 2.71 AND P= 2.03 INTERVALS

UNIT HYDROGRAPH 13 END-OF-PERIOD COORDINATES LAGE .77 HOURS, CP= .62 VOL= 1.00

MO.DA HR.MM PERIOD RAIN EXCS LOSS COVP Q MO.DA HR.MM PERIOD PAIN EXCS LOSS COMP Q

1.01	2.00	1	.00	.00	.00	0	1.02	1.20	76	.04	.01	.03	1
1.01	1.40	2	.00	.00	.00	0	1.02	1.40	77	.04	.01	.03	1
1.01	1.00	3	.00	.00	.00	0	1.02	2.00	78	.04	.01	.03	1
1.01	1.20	4	.00	.00	.00	0	1.02	2.20	79	.04	.01	.03	2
1.01	1.40	5	.00	.00	.00	0	1.02	2.40	80	.04	.01	.03	2
1.01	2.00	6	.00	.00	.00	0	1.02	3.00	81	.04	.01	.03	2
1.01	2.20	7	.00	.00	.00	0	1.02	3.20	82	.04	.01	.03	2
1.01	2.40	8	.00	.00	.00	0	1.02	3.40	83	.04	.01	.03	2
1.01	3.00	9	.00	.00	.00	0	1.02	4.00	84	.04	.01	.03	2
1.01	3.20	10	.00	.00	.00	0	1.02	4.20	85	.04	.01	.03	2
1.01	3.40	11	.00	.00	.00	0	1.02	4.40	86	.04	.01	.03	2
1.01	4.00	12	.00	.00	.00	0	1.02	5.00	87	.04	.01	.03	2
1.01	4.20	13	.00	.00	.00	0	1.02	5.20	88	.04	.01	.03	2
1.01	4.40	14	.00	.00	.00	0	1.02	5.40	89	.04	.01	.03	2
1.01	5.00	15	.00	.00	.00	0	1.02	6.00	90	.04	.01	.03	2
1.01	5.20	16	.00	.00	.00	0	1.02	6.20	91	.04	.01	.03	3
1.01	5.40	17	.00	.00	.00	0	1.02	6.40	92	.04	.01	.03	3
1.01	6.00	18	.00	.00	.00	0	1.02	7.00	93	.04	.01	.03	3
1.01	6.20	19	.00	.00	.00	0	1.02	7.20	94	.04	.01	.03	3
1.01	6.40	20	.00	.00	.00	0	1.02	7.40	95	.04	.01	.03	3
1.01	7.00	21	.00	.00	.00	0	1.02	8.00	96	.04	.01	.03	3
1.01	7.20	22	.00	.00	.00	0	1.02	8.20	97	.04	.01	.03	3
1.01	7.40	23	.00	.00	.00	0	1.02	8.40	98	.04	.01	.03	3
1.01	8.00	24	.00	.00	.00	0	1.02	9.00	99	.04	.01	.03	3
1.01	8.20	25	.00	.00	.00	0	1.02	9.20	100	.04	.01	.03	3
1.01	8.40	26	.00	.00	.00	0	1.02	9.40	101	.04	.01	.03	3
1.01	9.00	27	.00	.00	.00	0	1.02	10.00	102	.04	.01	.03	3
1.01	9.20	28	.00	.00	.00	0	1.02	10.20	103	.04	.01	.03	3
1.01	9.40	29	.00	.00	.00	0	1.02	10.40	104	.04	.01	.03	3
1.01	10.00	30	.00	.00	.00	0	1.02	11.00	105	.04	.01	.03	3
1.01	10.20	31	.00	.00	.00	0	1.02	11.20	106	.04	.01	.03	3
1.01	10.40	32	.00	.00	.00	0	1.02	11.40	107	.04	.01	.03	3
1.01	11.00	33	.00	.00	.00	0	1.02	12.00	108	.04	.01	.03	3
1.01	11.20	34	.00	.00	.00	0	1.02	12.20	109	.04	.01	.03	3
1.01	11.40	35	.00	.00	.00	0	1.02	12.40	110	.04	.01	.03	3
1.01	12.00	36	.00	.00	.00	0	1.02	13.00	111	.04	.01	.03	3
1.01	12.20	37	.00	.00	.00	0	1.02	13.20	112	.04	.01	.03	3
1.01	12.40	38	.00	.00	.00	0	1.02	13.40	113	.04	.01	.03	3
1.01	13.00	39	.00	.00	.00	0	1.02	14.00	114	.04	.01	.03	3
1.01	13.20	40	.00	.00	.00	0	1.02	14.20	115	.04	.01	.03	3
1.01	13.40	41	.00	.00	.00	0	1.02	14.40	116	.04	.01	.03	3
1.01	14.00	42	.00	.00	.00	0	1.02	15.00	117	.04	.01	.03	3
1.01	14.20	43	.00	.00	.00	0	1.02	15.20	118	.04	.01	.03	3

Sheet 2 of 25























PEAK FLOW AND STORAGE (END OF PERIOD) SUMMARY FOR MULTIPLE PLAN-RATIO ECONOMIC COMPUTATIONS  
 FLOWS IN CUBIC FEET PER SECOND (CUBIC METERS PER SECOND)  
 AREA IN SQUARE MILES (SQUARE KILOMETERS)

OPERATION	STATION	AREA	PLAN	RATIOS APPLIED TO FLOWS			
				RATIO 1	RATIO 2	RATIO 3	RATIO 4
				1.00	.75	.50	.25
HYDROGRAPH AT	1	.26 (.67)	1	.922 (26.12)	692. (19.59)	461. (13.06)	231. (6.53)
ROUTED TO	2	.26 (.67)	1	.922 (26.12)	690. (19.53)	456 (12.91)	143. (4.06)
ROUTED TO	3	.26 (.67)	1	.921 (26.07)	691. (19.55)	450 (12.73)	144. (4.08)
HYDROGRAPH AT	2	.09 (.23)	1	.363 (10.29)	273. (7.72)	182. (5.15)	91. (2.57)
2 COMBINED	2	.35 (.91)	1	1224. (34.66)	918. (25.99)	601. (17.02)	187. (5.30)
HYDROGRAPH AT	3	.11 (.28)	1	.433 (12.25)	324. (9.19)	216. (6.12)	108. (3.06)
2 COMBINED	3	.46 (1.19)	1	1647. (46.66)	1224. (34.66)	806. (22.83)	242. (6.85)
ROUTED TO	10	.46 (1.19)	1	1628. (46.10)	1222. (34.62)	837. (23.99)	159. (4.50)

# NORTH LAKE DAM

## SUMMARY OF DAM SAFETY ANALYSIS

PLAN 1		INITIAL VALUE		SPILLWAY CREST		TOP OF DAM	
ELEVATION		572.00		572.00		573.75	
STORAGE		210.		210.		249.	
OUTFLOW		0.		0.		70.	
RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1.00	574.37	.62	263.	923.	6.37	40.67	0.00
.75	574.25	.50	240.	800.	6.33	40.67	0.00
.50	574.11	.36	237.	456.	4.67	40.67	0.00
.25	573.86	.11	251.	143.	2.33	41.67	0.00

PLAN 1		STATION 3	
RATIO	MAXIMUM FLOW-CFS	MAXIMUM STAGE-FT	TIME HOURS
1.00	921.	478.7	40.67
.75	691.	478.3	40.67
.50	450.	477.8	40.67
.25	144.	477.0	41.67

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# Long Pond Dam SUMMARY OF DAM SAFETY ANALYSIS

PLAN 1									
ELEVATION		INITIAL VALUE		SPILLWAY CREST		TOP OF DAM			
STORAGE		470.00		470.00		476.00			
OUTFLOW		115.		115.		174.			
		0.		0.		891.			
RATIO OF PMF	MAXIMUM RESERVOIR W.S. ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF		TIME OF	
						MAX OUTFLOW		FAILURE	
						HOURS		HOURS	
1.00	476.22	.22	176.	1628.	2.67	40.67		0.00	
.75	476.11	.11	175.	1222.	1.67	40.33		0.00	
.50	475.66	0.00	173.	947.	0.00	40.57		0.00	
.25	474.30	0.00	155.	159.	0.00	42.67		0.00	

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FLOOD HYDROGRAPH PACKAGE (HEC-1)  
DAM SAFETY VERSION JULY 1978  
LAST MODIFICATION 01 APR 80  
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REFERENCES

APPENDIX E

#### REFERENCES

1. "Flood Hydrograph Package (HEC-1) Users Manual for Dam Safety Investigations", U. S. Army Corps of Engineers, Hydrologic Engineering Center, September 1979.
2. "Seasonal Variation of the Probable Maximum Precipitation, East of the 105th Meridian for Areas from 10 to 1,000 Square Miles, and Durations of 6, 12, 24 and 48 Hours", Hydrometeorological Report No. 33. Weather Bureau, U.S. Department of Commerce, April 1956.
3. "Recommended Guidelines for Safety Inspection of Dams", Department of the Army, Office of the Chief of Engineers, Appendix D.
4. The University of the State of New York, The State Education Department State Museum and Science Service Geological Survey - MAP and Chart Series No. 5, Geologic MAP of New York 1961, Lower Hudson Sheet.

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